# INDEX OF MISCELLANEOUS STANDARDS

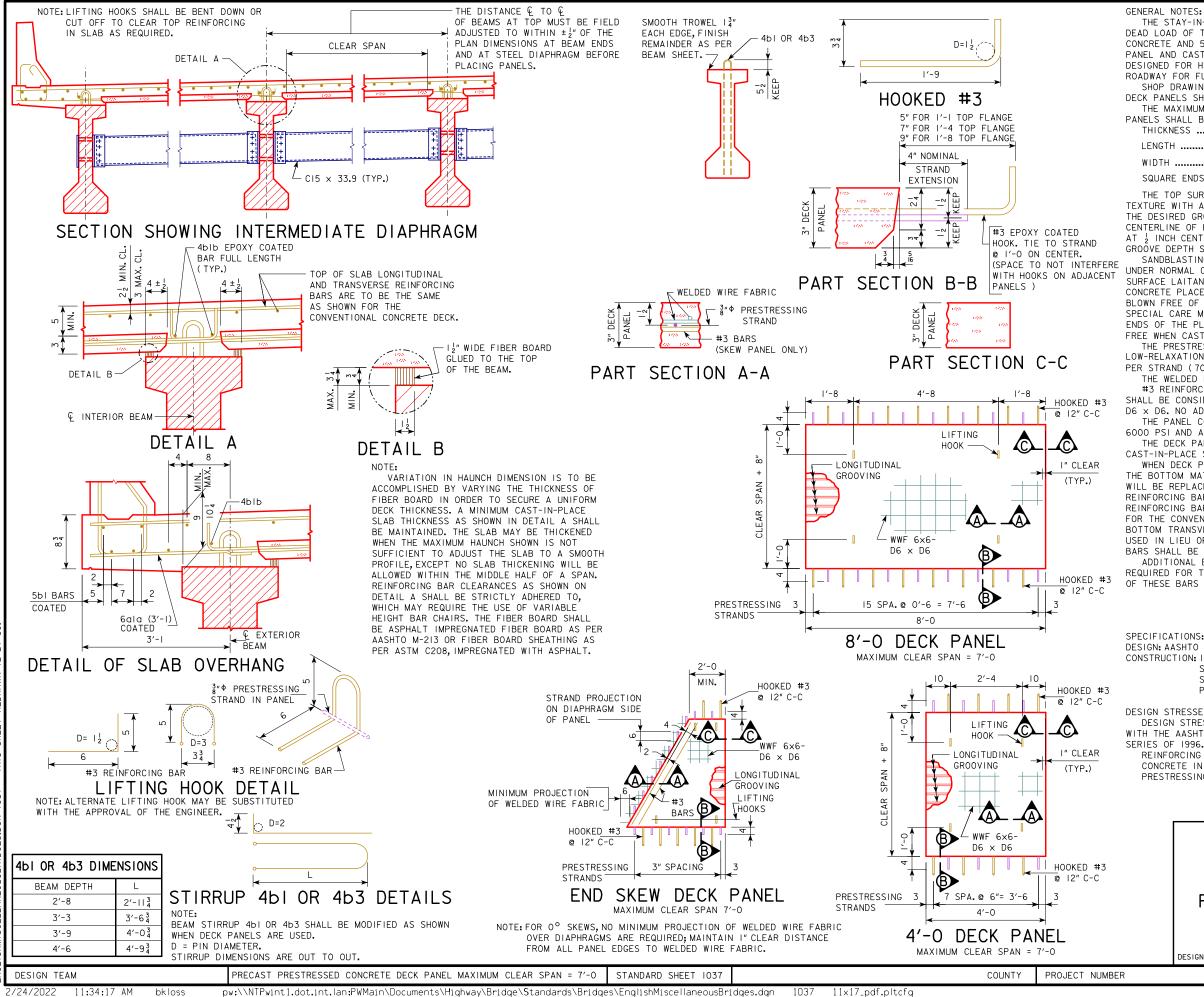
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1037A	PRECAST PRESTRESSED CONCRETE DECK PANEL
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1050	TEMPORARY BARRIER RAIL - F SHAPE CONCRETE - TWO WAY TRAFFIC
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PIOL	LRFD CONCRETE AND STEEL TRESTLE PILE BENTS

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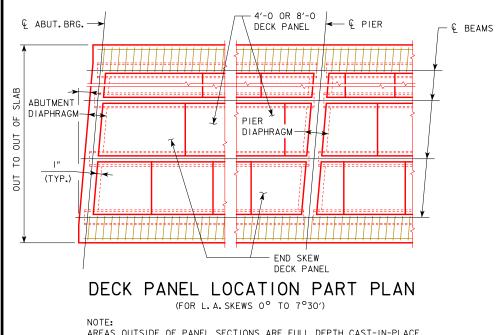
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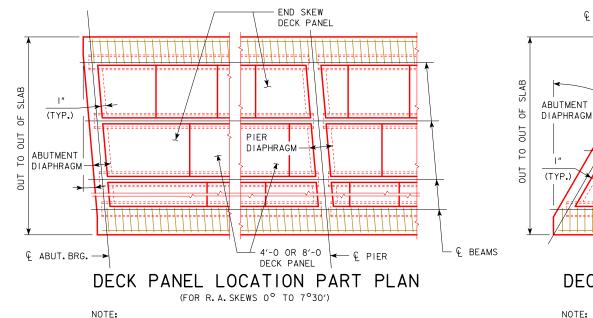
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THE STAY-IN-PLACE DECK PANELS ARE DESIGNED TO SUPPORT THE DEAD LOAD OF THE PANEL, REINFORCEMENT, PLASTIC CAST-IN-PLACE CONCRETE AND 50 LBS. PER SQUARE FOOT OF CONSTRUCTION LOAD. THE PANEL AND CAST-IN-PLACE SLAB, ACTING AS A COMPOSITE SECTION IS DESIGNED FOR HS20-44 LOADING PLUS 20 LBS.PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE. SHOP DRAWINGS SHOWING LAYOUT AND CONSTRUCTION DETAILS OF THE DECK PANELS SHALL BE SUBMITTED FOR APPROVAL. THE MAXIMUM ALLOWABLE DIMENSIONAL TOLERANCE FOR THE DECK PANELS SHALL BE AS FOLLOWS: THICKNESS ..... + "" OR -0" I ENGTH WIDTH ..... SQUARE ENDS (DEVIATION FROM SQUARE ) ..... ± 3 THE TOP SURFACE OF THE DECK PANELS SHALL BE GIVEN A SUITABLE TEXTURE WITH A WIRE BROOM OR COMB HAVING A SINGLE ROW OF TINES. THE DESIRED GROOVING IS LONGITUDINAL GROOVING (PARALLEL TO THE CENTERLINE OF BRIDGE ROADWAY ) WHICH MAY VARY FROM 16 INCH WIDTH AT 1 INCH CENTERS TO 16 INCH WIDTH AT 3 INCH CENTERS, AND THE GROOVE DEPTH SHOULD BE & INCH TO 16 INCH. SANDBLASTING THE PLANK SURFACE IS NOT CONSIDERED NECESSARY, UNDER NORMAL CONDITIONS, BUT MAY BE REQUIRED TO REMOVE UNUSUAL SURFACE LAITANCE OR OTHER SURFACE CONTAMINANTS. PRIOR TO CONCRETE PLACEMENT, THE PLANK SURFACE AND BEAM TOP SHALL BE BLOWN FREE OF DUST AND DEBRIS WITH AN OIL FREE AIR BLAST. SPECIAL CARE MUST BE TAKEN TO REMOVE ALL DEBRIS FROM UNDER THE ENDS OF THE PLANK. THE PLANK SURFACE SHALL BE DRY AND DUST FREE WHEN CAST-IN-PLACE CONCRETE IS PLACED ON THE PLANK. THE PRESTRESSING STRANDS SHALL BE ∦ ♥ GRADE 270 ASTM A416 LOW-RELAXATION STRANDS WITH AN INITIAL TENSION OF 16,100 LBS. PER STRAND (70% OF THE GUARANTEED ULTIMATE TENSILE STRENGTH.) THE WELDED DEFORMED STEEL WIRE FABILS SHALL BE ASTM A497. #3 REINFORCING BARS SPACED AT 1'-O CENTERS IN BOTH DIRECTIONS SHALL BE CONSIDERED AN ALLOWABLE SUBSTITUTION FOR THE WWF 6×6-D6 × D6. NO ADDITIONAL PAYMENT WILL BE PROVIDED. THE PANEL CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 6000 PSI AND A MINIMUM RELEASE STRENGTH OF 4500 PSI. THE DECK PANELS SHALL BE AT LEAST 28 DAYS OLD BEFORE THE CAST-IN-PLACE SLAB IS PLACED OR AS APPROVED BY ENGINEER. WHEN DECK PANELS ARE USED IN CONSTRUCTION OF BRIDGE DECK, THE BOTTOM MAT OF SLAB REINFORCING BARS BETWEEN ALL BEAMS WILL BE REPLACED BY CONCRETE DECK PANELS. THE BOTTOM LONGITUDINAL REINFORCING BARS IN THE SLAB OVERHANG AND THE TOP MAT OF REINFORCING BARS FOR THE SLAB ARE TO REMAIN THE SAME AS SHOWN FOR THE CONVENTIONAL FULL-DEPTH CAST-IN-PLACE SLAB, THE 6010 BOTTOM TRANSVERSE REINFORCING BARS IN THE SLAB OVERHANG SHALL BE USED IN LIEU OF THE 6aI BOTTOM TRANSVERSE REINFORCING BARS, 6ala BARS SHALL BE SPACED AND ORIENTED THE SAME AS Gal BARS. ADDITIONAL EPOXY COATED LONGITUDINAL BARS 4616 WILL ALSO BE REQUIRED FOR THE FULL LENGTH OF THE BRIDGE. THE LOCATION AND NUMBER OF THESE BARS IS SHOWN IN DETAIL A AND THE SLAB OVERHANG DETAIL. SPECIFICATIONS: DESIGN: AASHTO SERIES OF 1996. CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS, CURRENT SERIES, PLUS CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. DESIGN STRESSES: DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES. SERIES OF 1996. REINFORCING STEEL IN ACCORDANCE WITH SECTION 8. GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 9, f'c = 6,000 PSI. PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 9, f's = 270,000 PSI. PRECAST DECK PANEL DETAILS IOWA DEPARTMENT OF TRANSPORTATION DESIGN SHEET NO. OF FILE NO. DESIGN NO. SHEET NUMBER



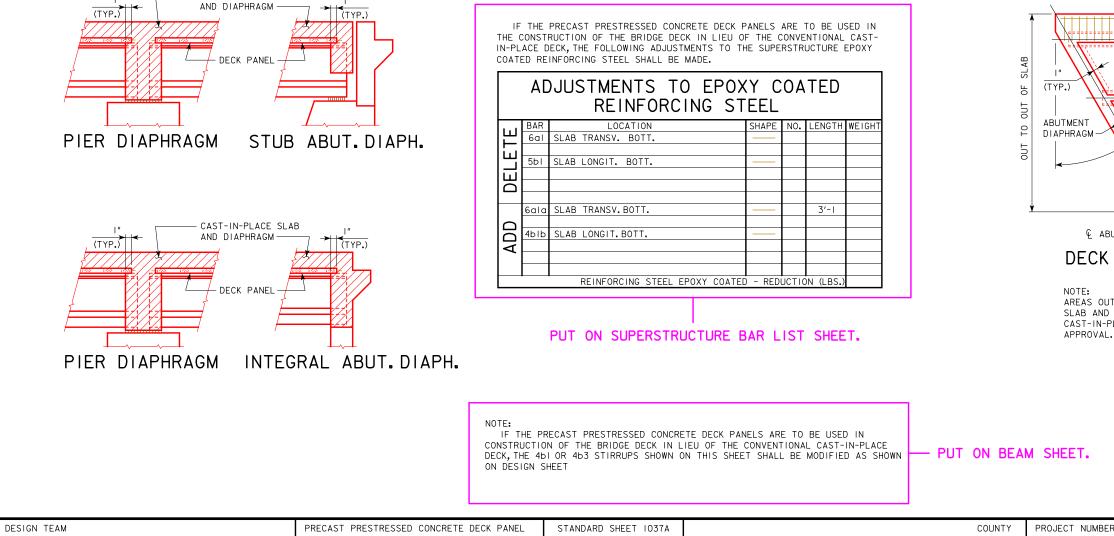
AREAS OUTSIDE OF PANEL SECTIONS ARE FULL DEPTH CAST-IN-PLACE SLAB AND DIAPHRAGMS. ALTERNATE DETAIL OF USING FULL DEPTH CAST-IN-PLACE SLAB AT THE SKEWED ENDS MAY BE SUBMITTED FOR APPROVAL.

CAST-IN-PLACE SLAB



AREAS OUTSIDE OF PANEL SECTIONS ARE FULL DEPTH CAST-IN-PLACE SLAB AND DIAPHRAGMS. ALTERNATE DETAIL OF USING FULL DEPTH CAST-IN-PLACE SLAB AT THE SKEWED ENDS MAY BE SUBMITTED FOR APPROVAL.





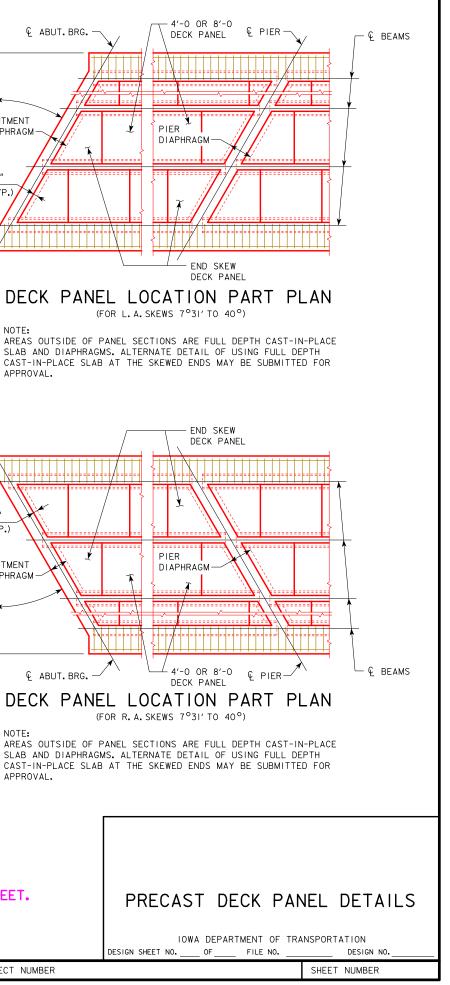
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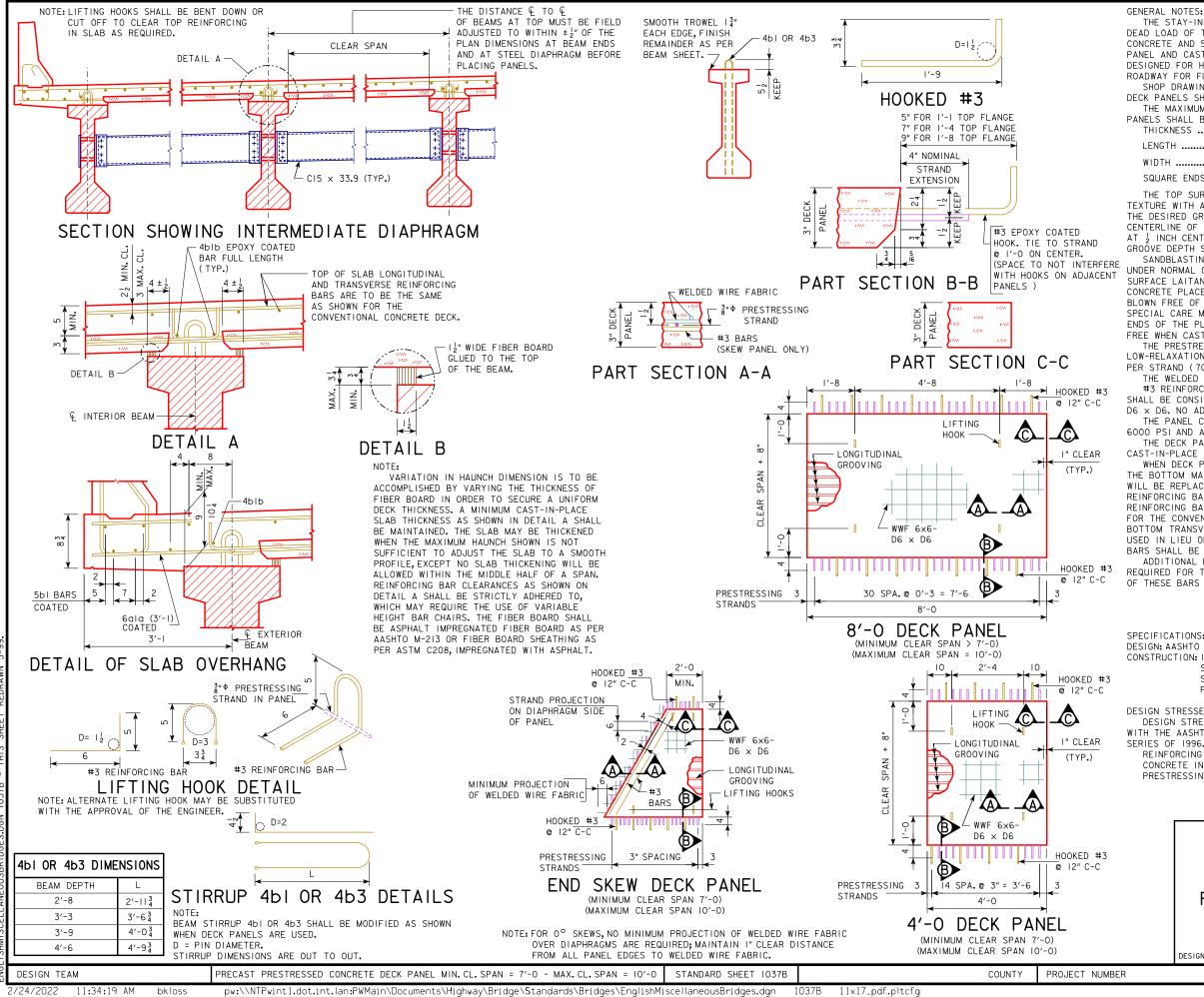
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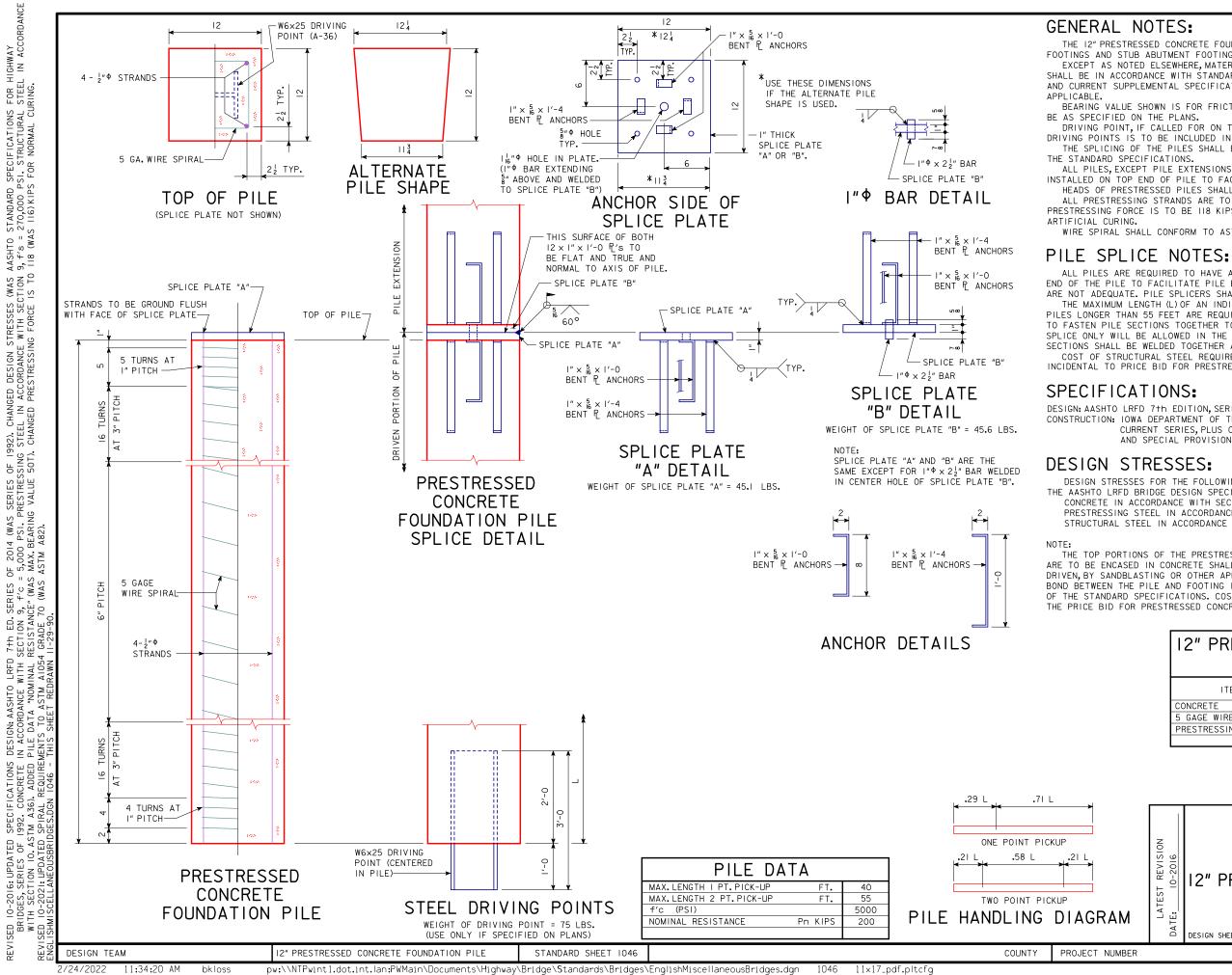
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AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY 9, f's = 270,000 PSI. STRUCTURAL STEEL IN ACCORDANCE 118 (WAS 116)KIPS FOR NORMAL CURING. DESIGN: AASHTO LRFD 7+h ED. SERIES OF 2014 (WAS SERIES OF 1992). CHANGED DESIGN STRESSES (WAS A IN ACCORDANCE WITH SECTION 9, f'c = 5,000 PSI. PRESTRESSING STEEL IN ACCORDANCE WITH SECTION PILE DATA "NOMINAL RESISTANCE" (WAS MAX. BEARING VALUE 50T). CHANGED PRESTRESSING FORCE IS TO WENTS TO ASTM A1054 GRADE 70 (WAS ASTM A82). THIS SHET REDRAWN 11-29-90. 10-2016: UPDATED SPECIFICATIONS IDGES, SERIES OF 1992. CONCRETE 1 FH SECTION 10. ASTM A361. ADBED 10-2021: UPDATED SPIRAL REQUIREN 15CELLANEOUSBRIDGES.DGN 1046 -

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THE 12" PRESTRESSED CONCRETE FOUNDATION PILE SHALL BE USED IN PIER FOOTINGS AND STUB ABUTMENT FOOTINGS ONLY.

EXCEPT AS NOTED ELSEWHERE, MATERIAL, CONSTRUCTION, DRIVING, AND EXTENSIONS SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS OF THE IOWA D.O.T. -AND CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS WHEN

BEARING VALUE SHOWN IS FOR FRICTION TYPE BEARING. BEARING VALUE SHALL

DRIVING POINT, IF CALLED FOR ON THE PLANS, SHALL BE AS DETAILED. COST OF ALL DRIVING POINTS IS TO BE INCLUDED IN THE PRICE BID PER LINEAL FOOT FOR PILING. THE SPLICING OF THE PILES SHALL BE IN ACCORDANCE WITH ARTICLE 2501.03, P, OF

ALL PILES, EXCEPT PILE EXTENSIONS IF REQUIRED, SHALL HAVE SPLICE PLATE "A" INSTALLED ON TOP END OF PILE TO FACILITATE SPLICING OF PILES AS NECESSARY. HEADS OF PRESTRESSED PILES SHALL BE NORMAL TO AXIS OF PILE. ALL PRESTRESSING STRANDS ARE TO BE 2" \$ 270K GRADE. THE TOTAL INITIAL PRESTRESSING FORCE IS TO BE 118 KIPS FOR NORMAL CURING OR 122 KIPS FOR

WIRE SPIRAL SHALL CONFORM TO ASTM A1064 GRADE 70.

ALL PILES ARE REQUIRED TO HAVE A PILE SPLICE PLATE "A" INSTALLED IN THE UPPER END OF THE PILE TO FACILITATE PILE EXTENSION IN THE EVENT THE PLAN LENGTH PILES ARE NOT ADEQUATE. PILE SPLICERS SHALL BE AS DETAILED ON THIS SHEET. THE MAXIMUM LENGTH (L) OF AN INDIVIDUAL SECTION OF PILE SHALL BE 55 FEET. WHEN PILES LONGER THAN 55 FEET ARE REQUIRED ON THE PLANS, PILE SPLICERS SHALL BE USED TO FASTEN PILE SECTIONS TOGETHER TO PROVIDE THE REQUIRED PLAN LENGTH. ONE PILE SPLICE ONLY WILL BE ALLOWED IN THE PLAN LENGTH OF PILES 56 TO 110 FEET. PILE SECTIONS SHALL BE WELDED TOGETHER AT SPLICES AFTER FIRST SECTION OF PILE IS DRIVEN. COST OF STRUCTURAL STEEL REQUIRED FOR SPLICE PLATES SHALL BE CONSIDERED INCIDENTAL TO PRICE BID FOR PRESTRESSED CONCRETE PILING - 12 INCH.

DESIGN: AASHTO LRFD 7th EDITION, SERIES OF 2014. CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS, CURRENT SERIES, PLUS CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS.

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 7th EDITION, SERIES OF 2014. CONCRETE IN ACCORDANCE WITH SECTION 5, f'c = 5,000 PSI. PRESTRESSING STEEL IN ACCORDANCE WITH AASHTO LRFD SECTION 5, f's = 270,000 PSI. STRUCTURAL STEEL IN ACCORDANCE WITH AASHTO LRFD SECTION 6. ASTM A709 GRADE 36.

THE TOP PORTIONS OF THE PRESTRESSED CONCRETE FOUNDATION PILES THAT ARE TO BE ENCASED IN CONCRETE SHALL BE ROUGHENED, AFTER PILES HAVE BEEN DRIVEN, BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE PILE AND FOOTING IN ACCORDANCE WITH ARTICLE 2403.03, 1, OF THE STANDARD SPECIFICATIONS. COST OF THIS WORK IS TO BE INCLUDED IN THE PRICE BID FOR PRESTRESSED CONCRETE PILING - 12 INCH.

12" PRESTR. CONC. MATERIAL C			
ITEM	UNIT	L=40'	ONE FOOT INCREMENT
CONCRETE	CU.YDS.	1.48	0.037
5 GAGE WIRE SPIRAL	LBS.	32	0.62

APPROVED BY:

PRESTRESSING STEEL

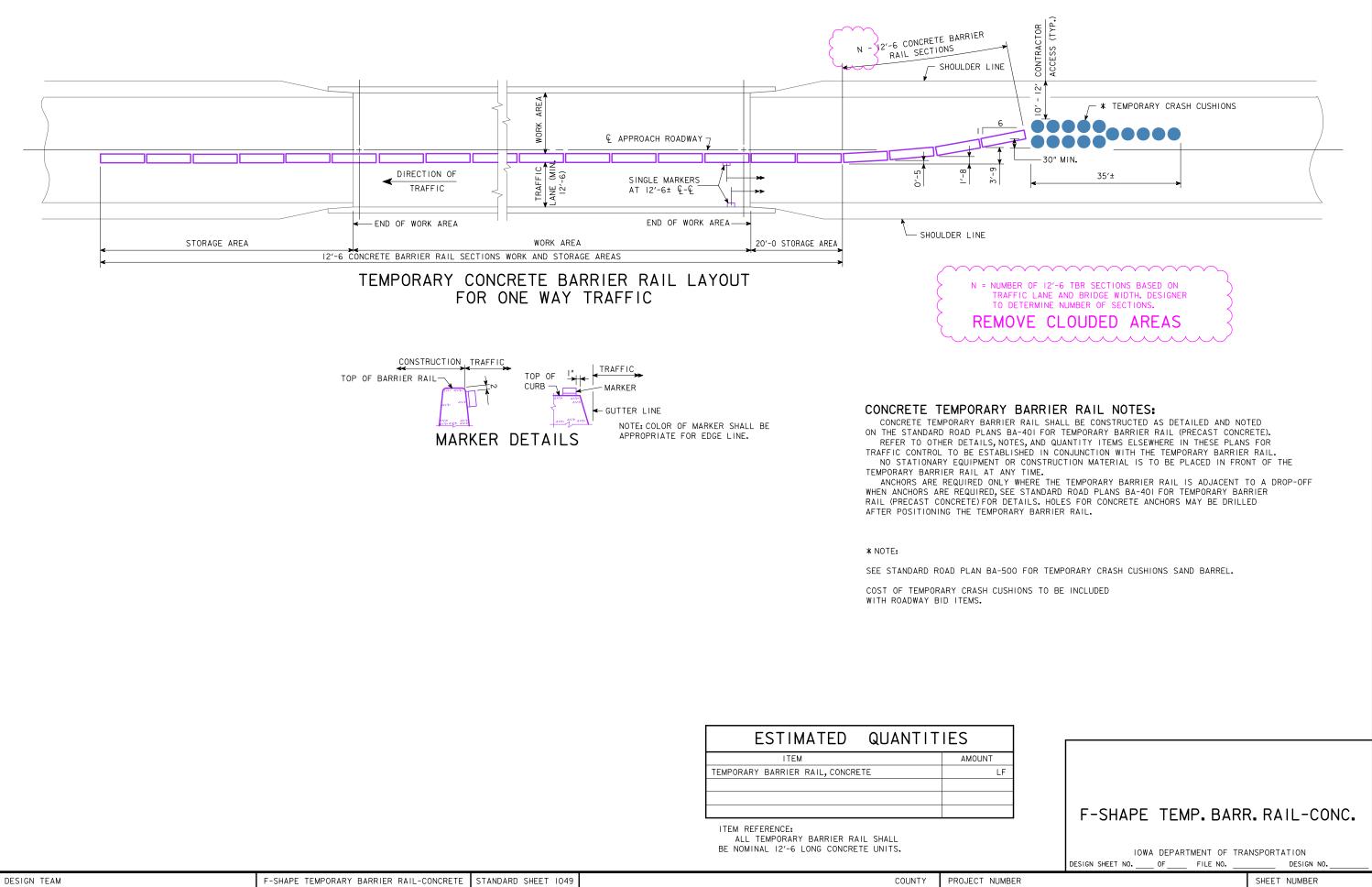
BRIDGE ENGINEER

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LBS.

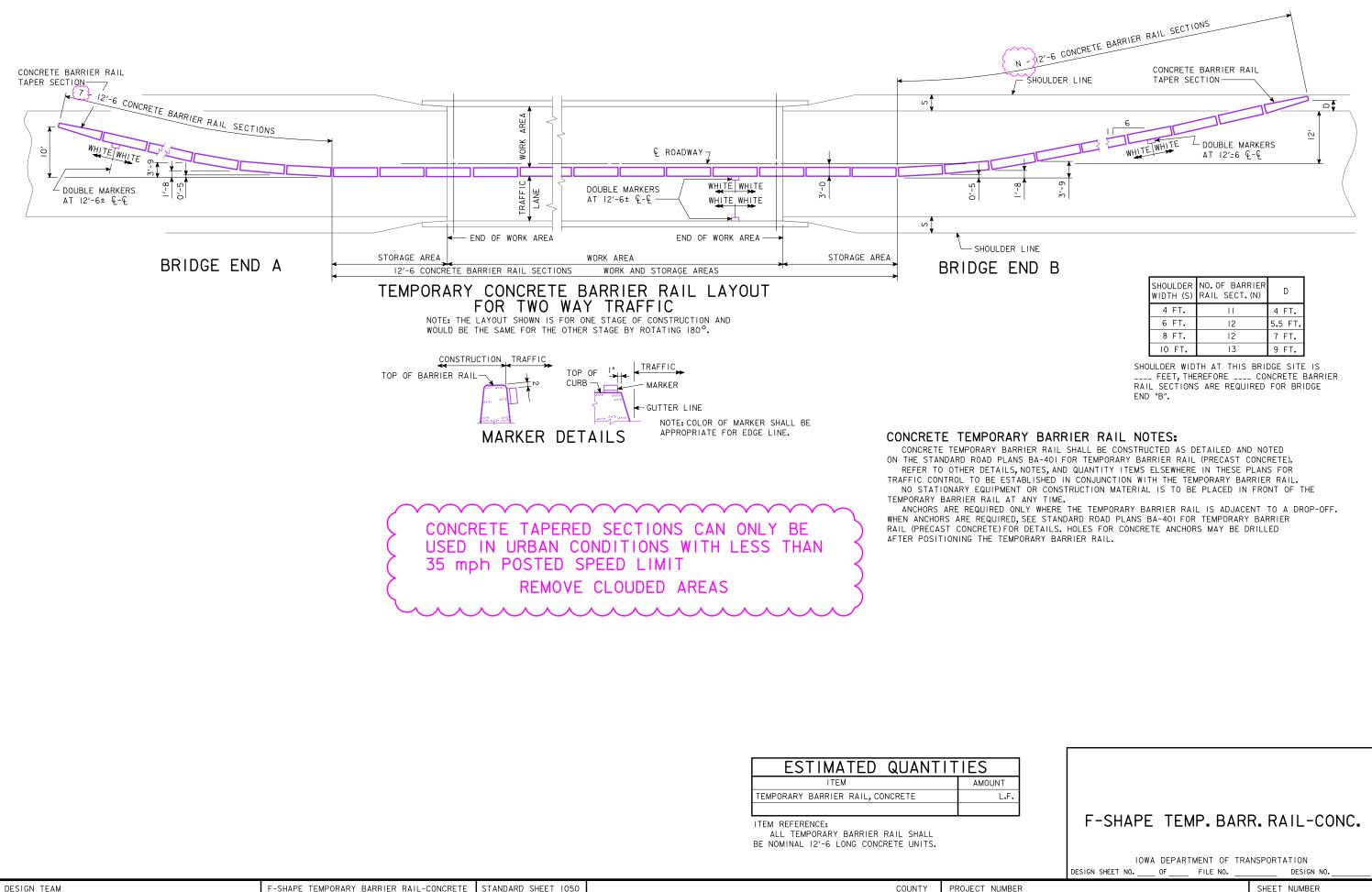
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	0	12" PRESTR. CONC. FOUNDATION PILES
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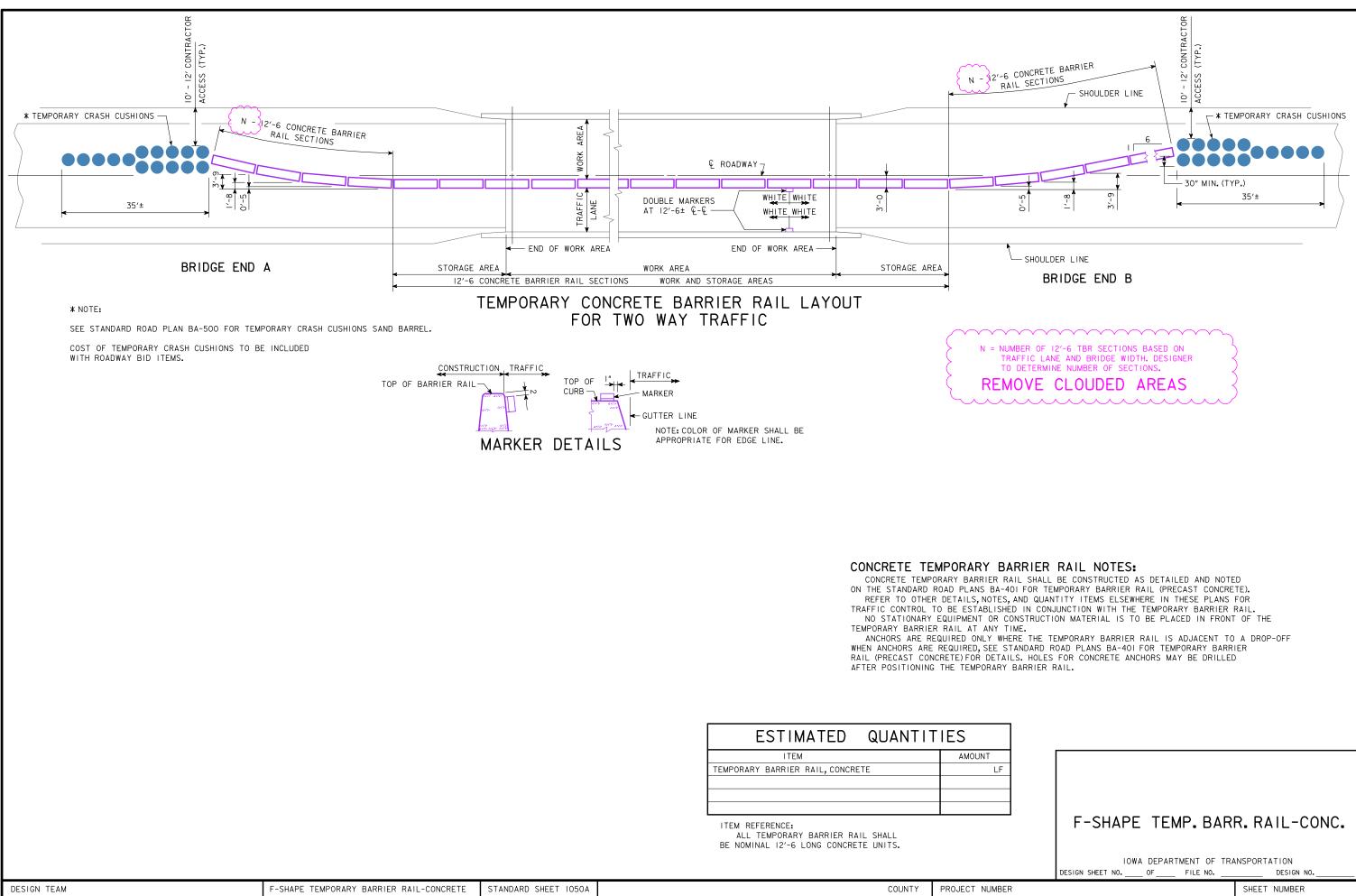
F-SHAPE TEMPORARY BARRIER RAIL-CONCRETE STANDARD SHEET 1050 COUNTY PROJECT NUMBER

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SHOULDER WIDTH (S)	NO.OF BARRIER RAIL SECT.(N)	D
4 FT.	П	4 FT.
6 FT.	12	5.5 FT.
8 FT.	12	7 FT.
IO FT.	13	9 FT.

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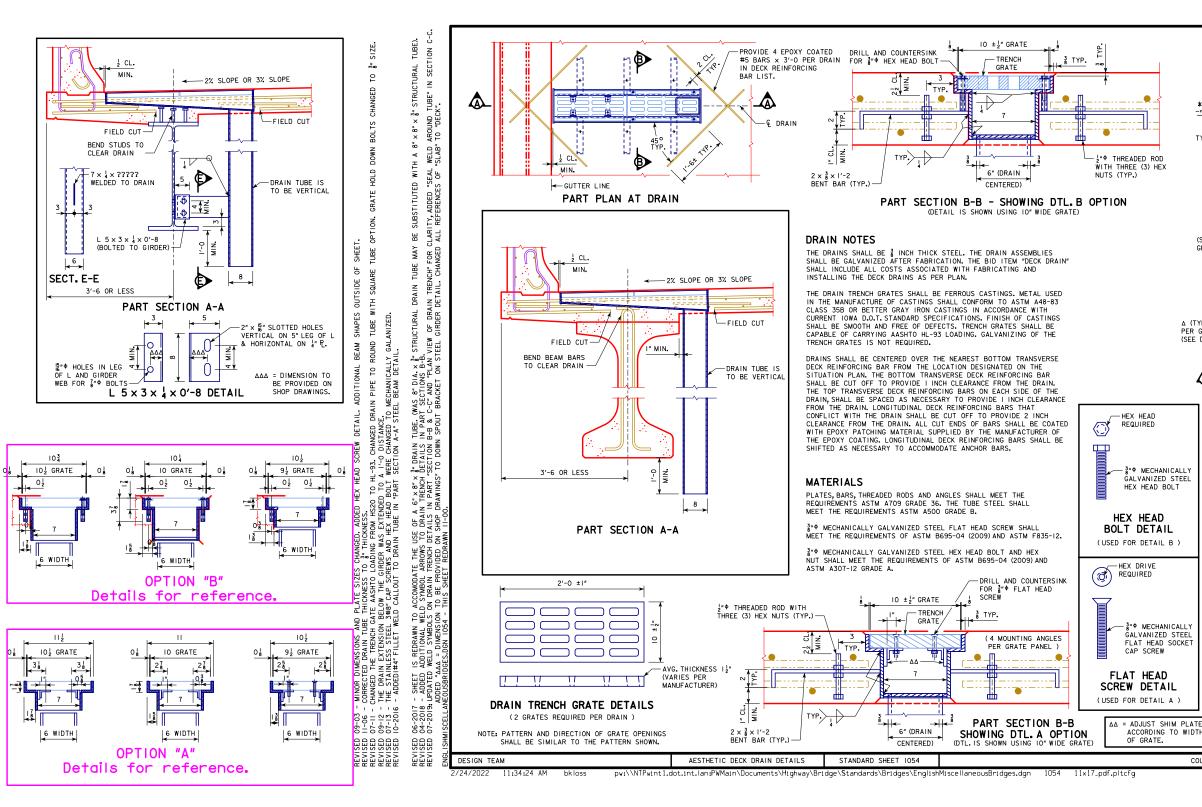
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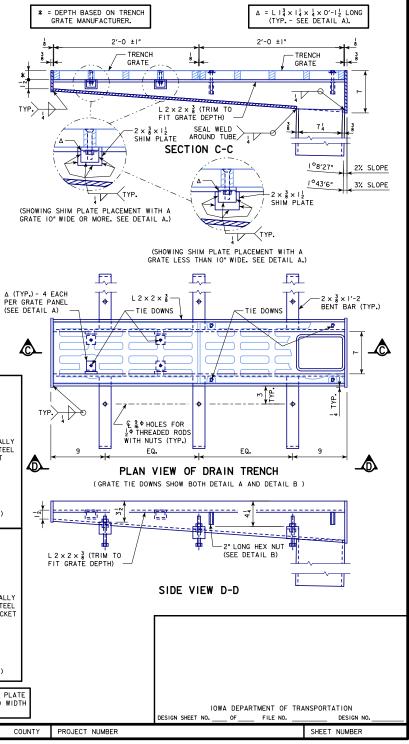
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## STEEL TEMPORARY BARRIER RAIL NOTES :

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**SECTIONS** 

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THE STEEL HPI4x73 TEMPORARY BARRIER RAILS SHALL BE CONSTRUCTED AS DETAILED AND NOTED ON THE STANDARD ROAD PLANS BA-400 FOR TEMPORARY BARRIER RAIL (STEEL).

HPI4x73 SECTIONS ARE TO BE JOINED BEFORE P.C. CONCRETE FILL IS PLACED. HP SECTIONS MAY BE JOINED BY BUTT WELDS ON BOTH EXTERIOR FACES AS DETAILED OR BY OTHER MEANS APPROVED BY THE ENGINEER. HP SECTIONS SHALL BE FREE FROM EXCESSIVE SWEEP AND CAMBER; STRAIGHTENING MAY BE REQUIRED BY THE ENGINEER IN ORDER TO PRODUCE A STABLE BARRIER.

CONCRETE MIX FOR THE P.C.FILL MAY BE ANY IOWA D.O.T. CONSTRUCTION SPECIFICATION MIX OR MAY BE A COMMERCIAL READY-MIX WITH A MINIMUM F'C = 2500 P.S.I. THE P.C.FILL MAY BE DEPOSITED BY A METHOD ACCEPTABLE TO THE ENGINEER. LIMITS OF FILL SHOWN ARE APPROXIMATE AND MAY BE ROUGH OR SLUMPED DEPENDING ON THE METHOD OF BULKHEADING.

REFER TO OTHER DETAILS, NOTES AND QUANTITY ITEMS, ELSEWHERE IN THESE PLANS FOR TRAFFIC CONTROL TO BE ESTABLISHED IN CONJUNCTION WITH THE TEMPORARY BARRIER RAIL.

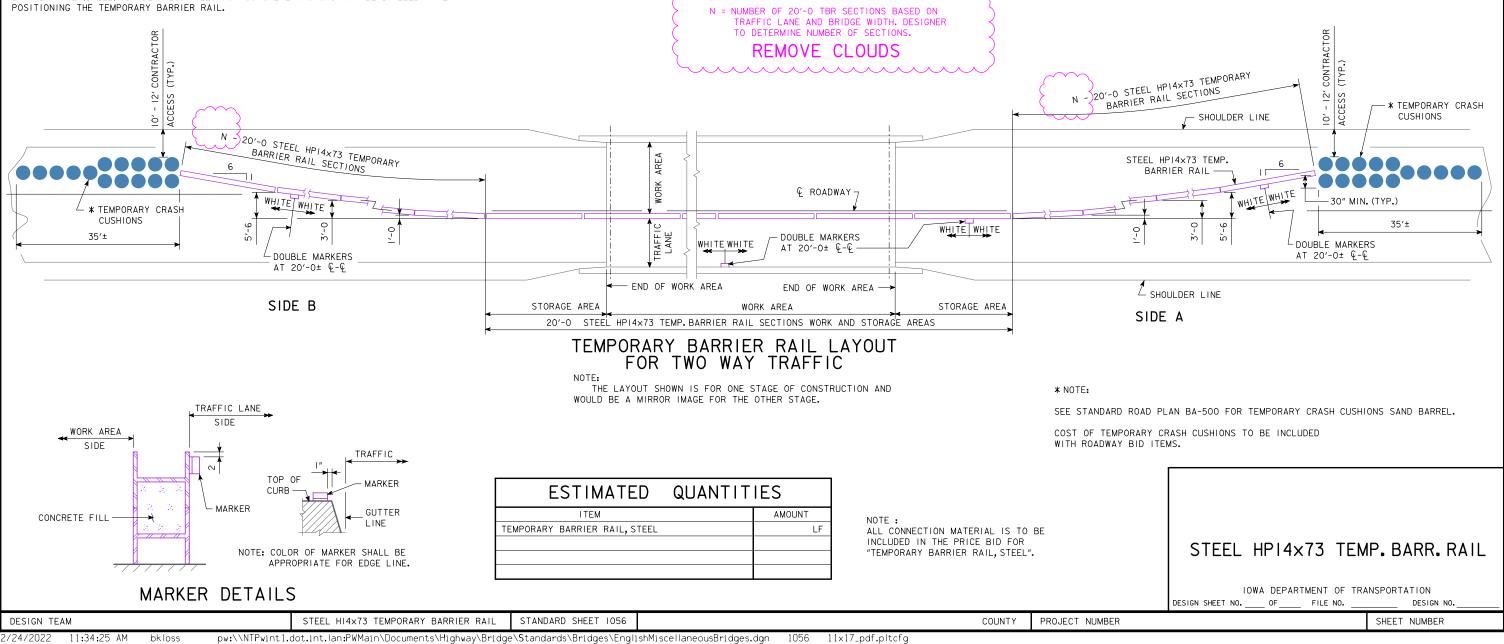
NO STATIONARY EQUIPMENT OR CONSTRUCTION MATERIAL IS TO BE PLACED IN FRONT OF THE TEMPORARY BARRIER RAIL AT ANY TIME. TRAFFIC MARKERS SHALL BE A RETRO-REFLECTIVE TYPE, IN ACCORDANCE WITH MATERIALS

INAFTIC MARKERS SHALL DE A REINU-REFLECTIVE TITE, IN ACCOMDANCE WITH MATERIALS I.M. 486.06. THEY SHALL BE LOCATED AS SHOWN ON THIS SHEET. THE CONTRACTOR SHALL MAINTAIN THE MARKERS AND SHALL PROMPTLY REPLACE ANY MISSING OR DAMAGED UNITS. ALL COSTS FOR FURNISHING, INSTALLING AND MAINTAINING MARKERS SHALL BE INCLUDED IN THE PRICE BID FOR "TEMPORARY BARRIER RAIL, STEEL".

CARE SHALL BE TAKEN IN MOVING THÉ STEEL TEMPORARY BARRIER RAIL FOR STAGE 2 CONSTRUCTION, SO THAT THE NEW CONCRETE OF STAGE I WILL NOT BE DAMAGED. ANY DAMAGE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

THE STEEL HP 14x73 TEMPORARY BARRIER RAIL IS TO BE BID ON A LINEAL FOOT BASIS. THE NUMBER OF LINEAL FEET OF TEMPORARY BARRIER RAIL INSTALLED WILL BE PAID FOR AT THE CONTRACT PRICE PER LINEAL FOOT BASED ON PLAN QUANTITIES. PRICE BID FOR "TEMPORARY BARRIER RAIL, STEEL" SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIAL, AND ALL OF THE EQUIPMENT AND LABOR REQUIRED TO ERECT THE RAIL IN ACCORDANCE WITH THESE PLANS AND CURRENT SPECIFICATIONS.

ANCHORS ARE REQUIRED ONLY WHERE THE TEMPORARY BARRIER RAIL IS ADJACENT TO A DROP-OFF. WHEN ANCHORS ARE REQUIRED, SEE STANDARD ROAD PLANS BA-400 FOR TEMPORARY BARRIER RAIL (STEEL) FOR DETAILS. HOLES FOR CONCRETE ANCHORS MAY BE DRILLED AFTER POSITIONING THE TEMPORARY BARRIER RAIL.



## STEEL TEMPORARY BARRIER RAIL NOTES :

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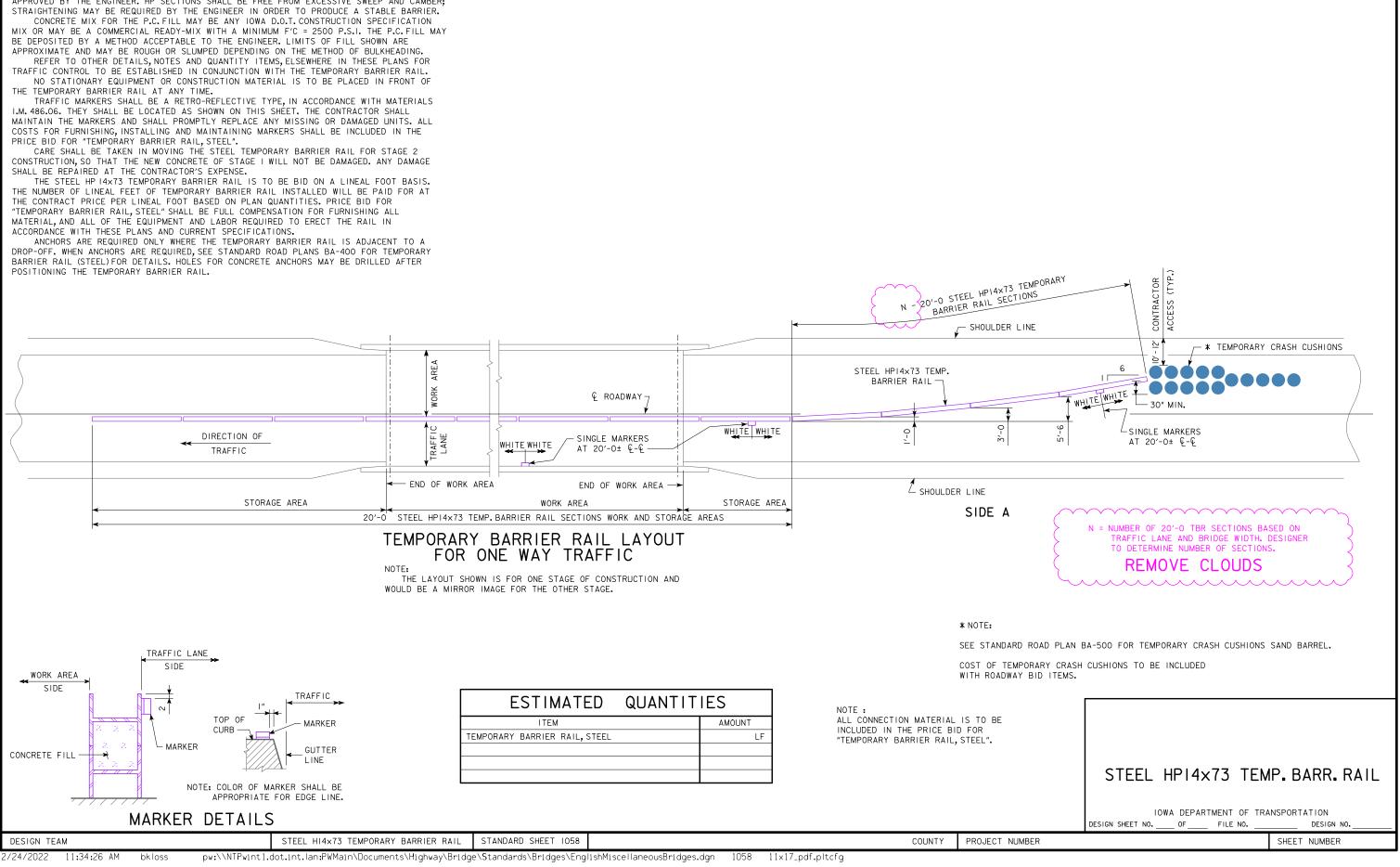
REFER TO OTHER DETAILS, NOTES AND QUANTITY ITEMS, ELSEWHERE IN THESE PLANS FOR TRAFFIC CONTROL TO BE ESTABLISHED IN CONJUNCTION WITH THE TEMPORARY BARRIER RAIL.

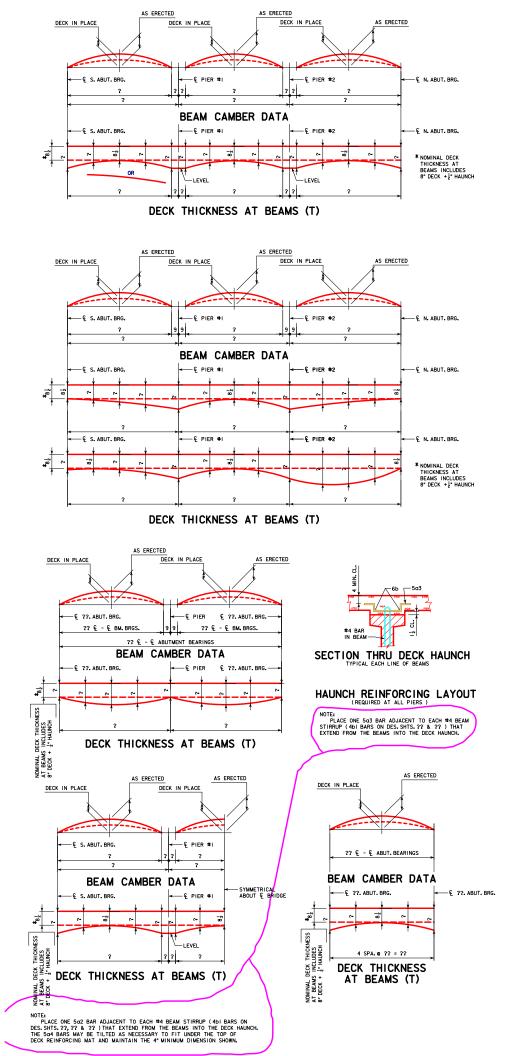
THE TEMPORARY BARRIER RAIL AT ANY TIME.

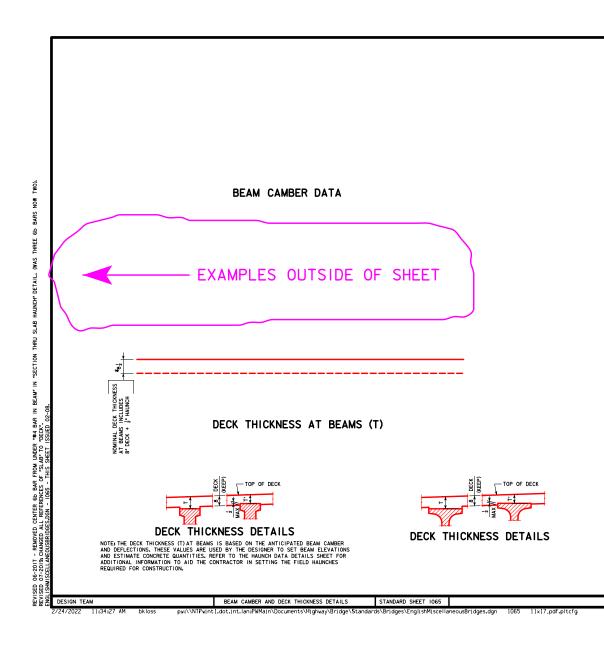
CARE SHALL BE TAKEN IN MOVING THE STEEL TEMPORARY BARRIER RAIL FOR STAGE 2 SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

"TEMPORARY BARRIER RAIL, STEEL" SHALL BE FULL COMPENSATION FOR FURNISHING ALL

ANCHORS ARE REQUIRED ONLY WHERE THE TEMPORARY BARRIER RAIL IS ADJACENT TO A







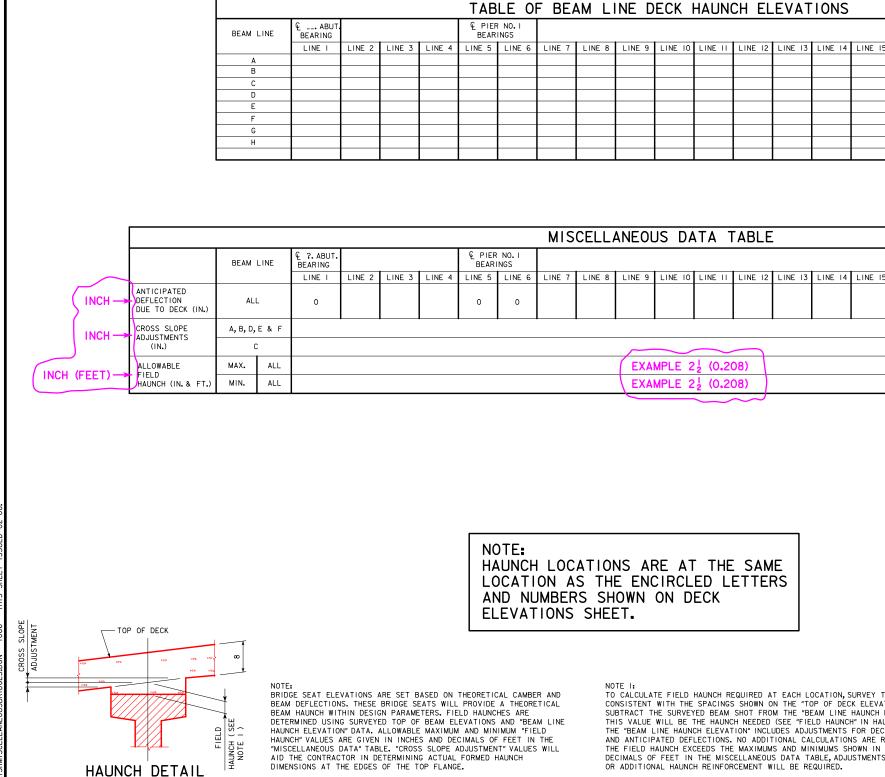
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### DECK THICKNESS DETAILS

IOWA DEPARTMENT OF TRANSPORTATION

COUNTY PROJECT NUMBER

SHEET NUMBER







HAUNCH

NOTE

HAUNCH DETAIL

DESIGN TEAM

2/24/2022

CROSS SLOPE ADJUSTMENT

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TOP OF DECK

144

HAUNCH DETAIL

- TOP OF DECK

HAUNCH DETAIL

COUNTY PR

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STANDARD SHEET 1066

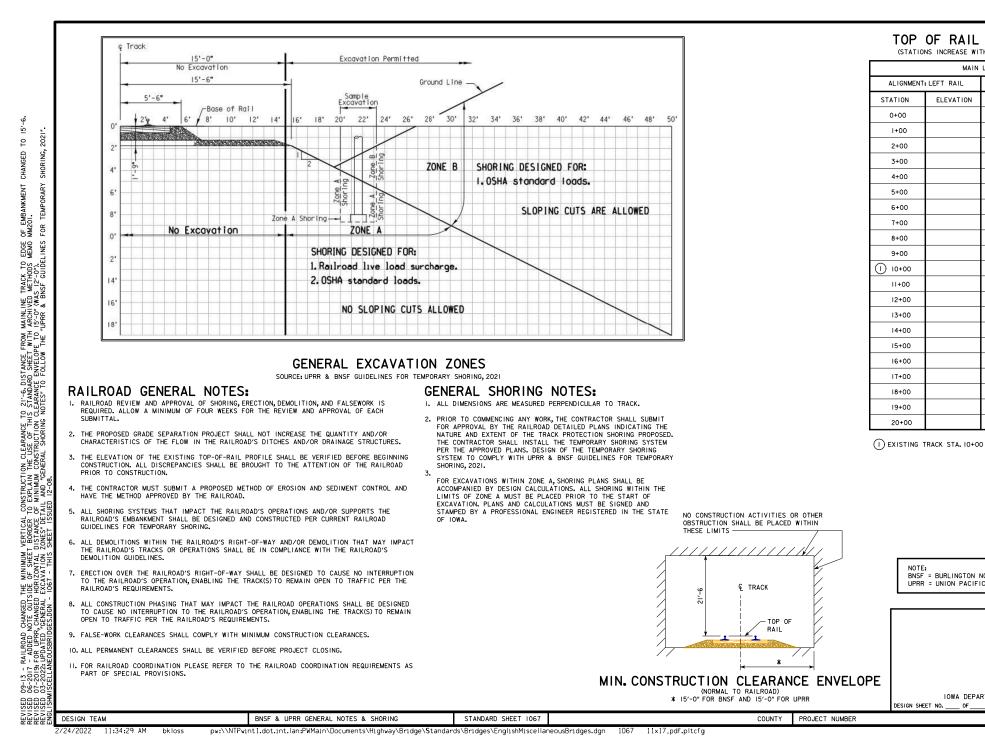
DECK HAUNCH DATA DETAILS

BENCH	MARK	N0.:

	€ PIER NO.2 BEARINGS					€ABUT. BEARING
15	LINE 16	LINE 17	LINE 18	LINE 19	LINE 20	LINE 21

	€ PIER BEAR	NO.2 INGS				€ ?.ABUT. BEARING
15	LINE 16	LINE 17	LINE 18	LINE 19	LINE 20	LINE 21
	0	0				0

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OJECT NUMBER		SHEET NUMBER



This information shown below is what is to be included on the TS&L sheet (Situation Plan) when this Standard Sheet 1067 is used. In discussions with the BNSF and UP railroads, the Bridge Bureau has agreed to provide the standard sheet 1067 and the information listed below. This information will be provided by Preliminary Bridge Design Unit on the Plan View and Elevation View on the TS&L sheet of all bridge projects that involve BNSF and UP railroad except the items noted with an asterisk (\*). These items will be provided by the Final Bridge Design Units. Final Design Units should review the list to make sure all information is provided. See archived Methods Memo MM201 for further explanation.

### Plan View

- I. Centerline of bridge and/or centerline of project. 2. Track layout and limits of railroad right-of-way with respect to centerline of main line. 5 main lines. 5. Future tracks, access roadways and existing tracks as main line, siding,
- spur, etc. 4. Horizontal clearance at right angle from centerline of nearest existing or future track to the face of obstruction such as substructure above grade. 45. Horizontal clearance at right angle from centerline of nearest existing or future track to the face of nearest foundation below grade. 6. Horizontal spacing at right angle between centerlines of existing and/or future tracke

- uture tracks. **\$7.** Limits of shoring and minimum distance at right angle from centerline of
- earest track. . All existing facilities and utilities.
- Existing ground shots and proposed grading. D. Railroad Milepost and direction of increasing Milepost (Provided by

- tairoad, 1. Direction of flow for all drainage systems within project limits, kl2. Limits of barrier rall and fence with respect to centerline of track, kl3. Location of deck drains (Note drains shall not be located over the ailroad right-of-way).
- kl4. Total width of superstructure.
- 15. Width of shoulder and/or sidewalk.
  16. North arrow
  17. Footprint of proposed superstructure and substructure including existing structure if applicable

### Elevation View

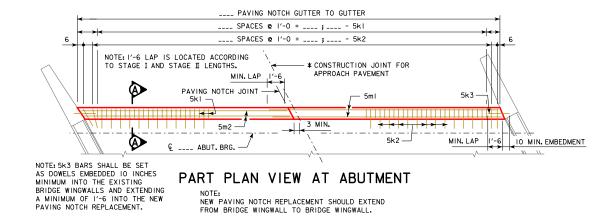
- I. Future tracks, access roadways and existing tracks as main line, siding,
- spur, etc. 2. Point of minimum vertical clearance and distance within the vertical clearance envelope, measured perpendicular from the centerline of nearest
- **\*3.** Limits of shoring and minimum distance at right angle from centerline of

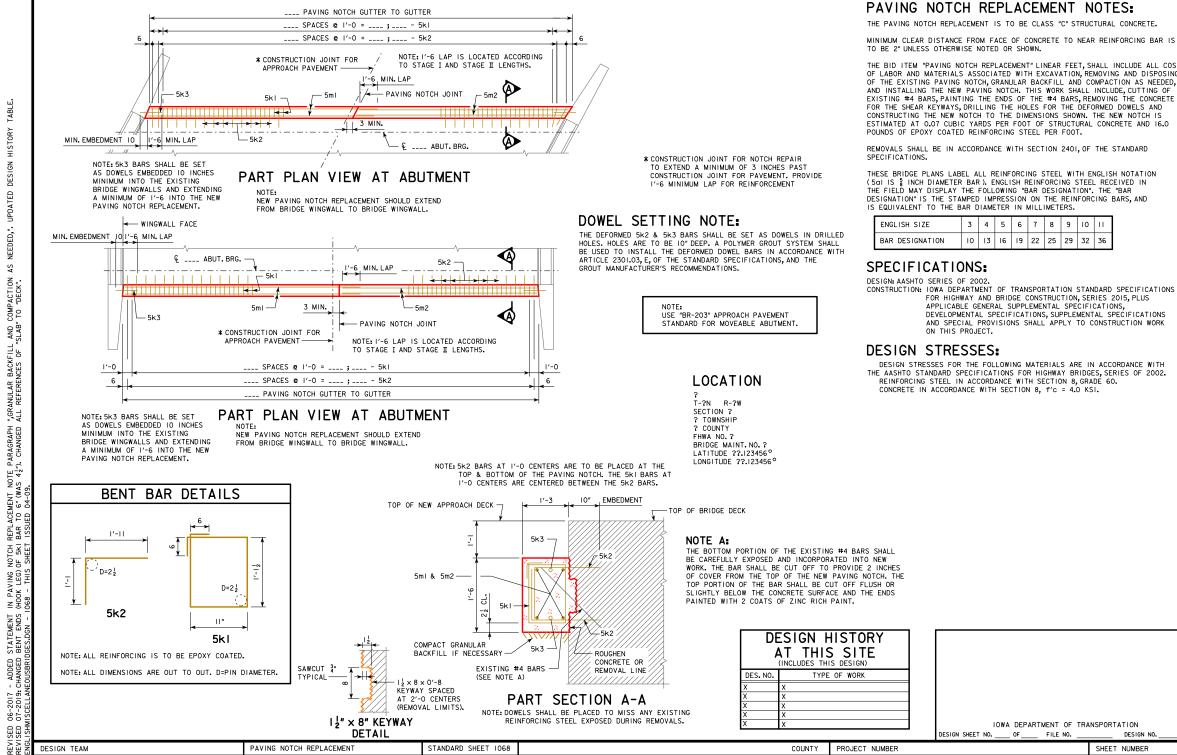
- 4.5. Limits of storing and minimum actions of retaining wall.
  4. Toe of slope and/or limits of retaining wall.
  5. Limits of barrier rail and fence with respect to centerline of track.
  6. Depth of foundation from top of tie / base of rail.
  4. Top and bottom of pier protection wall elevation relative to top of rail

- elevation. 8. Controlling dimensions of drainage ditches and/or drainage structures. 9. Top of rail elevations for all tracks. 10. Minimum permanent vertical clearance above the top of high rail to the lowest point under the bridge. 11. Existing and proposed groundline and roadway profile. 12. Show slope and specify type of slope paving. Toe of slope shall be shown relative to drainage ditch and top of subgrade.

/I <sup>-</sup>	ELEVAT		
N	LINE		
	ALIGNMENT	RIGHT RAIL	
	STATION	ELEVATION	
	0+00		
	1+00		
	2+00		
	3+00		
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PARTMENT OF TRANSPORTATION 	
SHEET NUMBER	





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STANDARD SHEET 1068

PAVING NOTCH REPLACEMENT

DESIGN TEAM

COUNTY PROJECT NUMBER

THE PAVING NOTCH REPLACEMENT IS TO BE CLASS "C" STRUCTURAL CONCRETE.

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

THE BID ITEM "PAVING NOTCH REPLACEMENT" LINEAR FEET, SHALL INCLUDE ALL COSTS OF LABOR AND MATERIALS ASSOCIATED WITH EXCAVATION, REMOVING AND DISPOSING OF THE EXISTING PAVING NOTCH, GRANULAR BACKFILL AND COMPACTION AS NEEDED. OF THE EXISTING PAVING NOTCH, GRANULAR BACKFILL AND COMPACTION AS NEEDED, AND INSTALLING THE NEW PAVING NOTCH. THIS WORK SHALL INCLUDE, CUTTING OF EXISTING #4 BARS, PAINTING THE ENDS OF THE #4 BARS, REMOVING THE CONCRETE FOR THE SHEAR KEYWAYS, DRILLING THE HOLES FOR THE DEFORMED DOWELS AND CONSTRUCTING THE NEW NOTCH TO THE DIMENSIONS SHOWN. THE NEW NOTCH IS ESTIMATED AT 0.07 CUBIC YARDS PER FOOT OF STRUCTURAL CONCRETE AND 16.0

(Sai IS & INCH DIAMETER BAR), ENGLISH REINFORCING STEEL RECEIVED IN THE FIELD MAY DISPLAY THE FOLLOWING "BAR DESIGNATION", THE "BAR DESIGNATION" IS THE STAMPED IMPRESSION ON THE REINFORCING BARS, AND IS EQUIVALENT TO THE BAR DIAMETER IN MILLIMETERS.

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16	19	22	25	29	32	36

FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2015, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2002. REINFORCING STEEL IN ACCORDANCE WITH SECTION 8, GRADE 60.

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				SHEET	NUMBER	

# HYDRODEMOLITION NOTES:

IT IS THE WITENT TO USE THE HYDRODEMOLITION PROCESS TO REMOVE ALL UNSOUND CONCRETE AS DIRECTED BY THE ENGINEER BY USING A HIGH PRESSURE WATER STREAM. REMOVALS SHALL SREATE A VERY ROUGH, BONDABLE SURFACE FOR NEW CONCRETE TO ADHERE TO. ALL HMA PATCH MATERIAL SHALL BE REMOVED PRIOR TO HYDRODEMOLITION. CONCRETE PATCH MATERIAL CAN REMAIN IF DETERMINED TO BE SOUND.

THE COMPLETE BRIDGE DECK SURFACE SHALL BE MILLED TO A DEPTH OF  $\frac{1}{4}$  INCH BEFORE HYDRODEMOLITION BEGINS.

THE CONTRACTOR SHALL USE SELF-PROPERLED HYDRODEMOLITION EQUIPMENT THAT PROVIDES A HIGH PRESSURE WATER JET STREAM FOR CONCRETE REMOVALS. THIS EQUIPMENT SHALL BE CAPABLE OF REMOVING ANY UNSOUND CONCRETE, AS WELL AS REMOVING RUST AND CONCRETE PARTICLES FROM EXPOSED REINFORCING STEEL.

CLEAN POTABLE WATER SHALL BE PROVIDED FOR HIGH PRESSURE REMOVALS AND THE EXCESS WATER RECLAIMED USING VACUUM METHODS OF COLLECTION, THEN FILTERED AND REUSED AS MUCH AS PRACTICAL.

OPERATION OF THE HYDRODEMOLITION EQUIPMENT SHALL BE PERFORMED AND SUPERVISED BY QUALIFIED PERSONNEL CERTIFIED BY THE EQUIPMENT MANUFACTURER.

THE CONTRACTOR SHALL ENSURE CONTAINMENT OF ALL DISLODGED MATERIAL, FLYING DEBRIS AND EXCESS WATER WITHIN THE EXISTING ROADWAY AND NOT ALLOW DEBRIS OR WATER TO ENTER ADJACENT TRAVEL LANES OR TRAFFIC, OR BELOW THE WORK AREA.

# CALIBRATING HYDRODEMOLITION:

PRIOR TO THE COMMENCEMENT OF THE REMOVAL OPERATION WITH HYDRODEMOLITION, THE EQUIPMENT SHALL BE CALIBRATED ON TWO SECTIONS DESIGNATED BY THE ENGINEER TO DEMONSTRATE THAT EQUIPMENT, PERSONNEL AND METHODS OF OPERATION ARE CAPABLE OF PRODUCING RESULTS SATISFACTORY TO THE ENGINEER. THE TRIAL SECTIONS WILL BE APPROXIMATELY 100 SQUARE FEET EACH, CONSISTING OF ONE SECTION OF SOUND CONCRETE THEN ONE SECTION OF DETERIORATED CONCRETE. THE CALIBRATION SHALL NOT INCLUDE ANY AREAS OF EXISTING OVERLAY OR PATCH MATERIAL. THE CONTRACTOR WILL DOCUMENT THE FOLLOWING INITIAL SETTINGS:

I. WATER PRESSURE GAUGE (13,000 PSI MINIMUM)

- 2. WATER USAGE (55 GALLONS PER MINUTE, MINIMUM) VERIFY NEED FOR THIS VALUE
- MACHINE STAGING CONTROL (STEP)
- 4. NOZZLE SIZE 5. NOZZLE SPEED (TRAVEL )

AFTER THE INITIAL TEST ON SOUND CONCRETE, THE EQUIPMENT SHALL THEN BE MOVED TO THE DETERIORATED AREA TO VERIFY THAT INITIAL SETTINGS WILL FULLY REMOVE UNSOUND CONCRETE WITHIN THE DESIGNATED AREA. THE INITIAL SETTINGS MAY NEED TO BE ADJUSTED, WITHIN THE LIMITS ESTABLISHED ABOVE, IN ORDER TO ACHIEVE TOTAL REMOVAL OF UNSOUND CONCRETE. THE CONTRACTOR SHALL DOCUMENT THE FINAL EQUIPMENT SETTINGS RESULTING FROM THE CALIBRATION PROCESS.

# CONCRETE BRIDGE DECK REMOVAL BY HYDRODEMOLITION:

AFTER CALIBRATION OF THE EQUIPMENT, CONCRETE REMOVAL BY HYDRODEMOLITION SHALL BE CONDUCTED ON THE BRIDGE DECK. THE REMOVAL SETTINGS WILL BE VERIFIED AS NECESSARY. THE EQUIPMENT SETTINGS WILL BE DOCUMENTED BY THE CONTRACTOR AND PROVIDED TO THE ENGINEER. CALIBRATION OF THE HYDRODEMOLITION EQUIPMENT SHALL BE CONDUCTED FOR EVERY DAY OF OPERATION AND, IF NECESSARY, RE-CALIBRATED TO INSURE REMOVAL OF KNOWN AREAS OF DELAMINATED CONCRETE AS WELL AS TO GUARD AGAINST EXCESSIVE REMOVAL OF SOUND CONCRETE. HANDCHIPPING MAY BE USED IN AREAS THAT ARE INACCESSIBLE TO THE SELF-PROPELLED OR HAND OPERATED HYDRODEMOLITION EQUIPMENT. HANDCHIPPING TOOLS (15 LBS. MAXIMUM) MAY BE HAND OR MECHANICALLY DRIVEN.

# ADDITIONAL REMOVAL:

AFTER CONCRETE BRIDGE DECK REMOVAL BY HYDRODEMOLITION HAS BEEN COMPLETED FOR THE CONSTRUCTION PHASE, THE DECK WILL UNDERGO FINAL SOUNDING TO ASSURE THAT ALL UNSOUND CONCRETE HAS BEEN REMOVED. THE PREPARED DECK SURFACE WILL BE COMPLETELY DRY PRIOR TO FINAL SOUNDING AND WILL CONSIST OF AS MANY SUCCESSIVE SOUNDINGS AS REQUIRED TO ENSURE THAT ALL DELAMINATED OR DEBONDED CONCRETE HAS BEEN REMOVED, ADDITIONAL CONCRETE REMOVAL SHALL BE PERFORMED BY HANDCHIPPING AND/OR HYDRODEMOLITION. HANDCHIPPING TOOLS MAY BE HAND OR MECHANICALLY DRIVEN AND OPERATED IN ACCORDANCE WITH ARTICLE 2413.03 OF THE STANDARD SPECIFICATIONS.

IN ADDITION, WHERE REINFORCING STEEL IS EXPOSED AND CONCRETE AND STEEL ARE NO LONGER BONDED, REMOVE ANY CONCRETE TO CLEAR AT LEAST  $\frac{3}{4}$ " INCH AROUND THE EXPOSED BARS. UN-BONDED BARS SHALL DETERMINED BY THE ENGINEER, MORE THAN ONE-HALF OF THE BAR PERIMETER MAY BE EXPOSED AND STILL DETERMINED TO BE "BONDED". DO NOT USE CHIPPING HAMMERS HEAVIER THAN 15 LBS. TO REMOVE CONCRETE. EXTREME CARE SHALL BE TAKEN TO ENSURE THAT NO DAMAGE IS DONE TO ANY REINFORCING BARS EXPOSED DURING THE REMOVAL PROCESS. ANY DAMAGE DONE SHALL BE REPAIRED BY THE CONTRACTOR AS APPROVED BY THE ENGINEER AT NO ADDITIONAL COST TO THE STATE.

# FULL DEPTH REPAIR OF BRIDGE DECK:

WHERE THE DECK IS SOUND FOR LESS THAN HALF OF ITS ORIGINAL DEPTH, THE CONCRETE SHALL BE REMOVED FULL DEPTH (DESIGNATED AS CLASS B REPAIR) EXCEPT FOR LIMITED AREAS AS DETERMINED BY THE ENGINEER.

# PREPARATION OF BRIDGE DECK PRIOR TO OVERLAY PLACEMENT:

VACUUMING OF DEBRIS AND WATER SHALL BE DONE IMMEDIATELY AFTER ANY HYDRODEMOLITION WORK. CONTRACTOR IS TO ENSURE ALL WATER RUN-OFF AND RESIDUAL MATERIAL IS CONTAINED WITHIN THE WORK AREA AND COLLECTED FOR DISPOSAL. DISPOSE OF EXCESS WATER AND DEBRIS AS APPROVED BY THE ENGINEER.

CLEANING OF THE HYDRODEMOLITION DEBRIS AND SLURRY SHALL BE PERFORMED WITH A VACUUM SYSTEM EQUIPPED WITH DUST CONTROL DEVICES AND CAPABLE OF REMOVING WET DEBRIS AND WATER IN THE SAME PASS. THE VACUUM EQUIPMENT SHALL BE CAPABLE OF WASHING THE DECK WITH PRESSURIZED WATER DURING THE VACUUM OPERATION TO DISLODGE ALL DEBRIS AND SLURRY FROM THE BRIDGE DECK SURFACE. CLEANING SHALL BE DONE BEFORE DEBRIS AND SLURRY IS ALLOWED TO DRY ON THE BRIDGE DECK SURFACE.

AFTER COMPLETION OF HYDRODEMOLITION AND ADDITIONAL REMOVALS, BUT NOT MORE THAN 24 HOURS PRIOR TO PLACEMENT OF THE OVERLAY, THE ENTIRE DECK SHALL BE SANDBLASTED OR WATER BLASTED TO EXPOSE FINE AND COARSE AGGREGATES AND TO REMOVE LAITANCE FROM THE SURFACE, EXPOSED REINFORCING STEEL AND THE CONCRETE UNDER AND AROUND THE EXPOSED STEEL SHALL BE THOROUGHLY CLEANED BY SANDBLASTING OR WATER BLASTING. THE SURFACE SHALL BE CLEANED USING COMPRESSED AIR TO REMOVE ALL DUST, CHIPS AND WATER. AIR LINES FOR SANDBLASTING AND COMPRESSED AIR CLEANING SHALL BE EQUIPPED WITH OIL TRAPS.

# BID ITEM INFORMATION:

THE BID TEM "HYDRODEMOLITION REMOVAL" SHALL INCLUDE ALL COSTS FOR HYDRODEMOLITION, CLEAN-UP, WATER CONTROL, DISPOSAL, AND FINAL CLEAN-UP IN PREPARATION FOR "CONCRETE REPAIR, REPLACE VARIABLE DEPTH CONCRETE".

THE BID ITEM "CONCRETE REPAIR, REPLACE VARIABLE DEPTH CONCRETE", CUBIC YARDS, SHALL INCLUDE THE ADDITIONAL CONCRETE TO REPAIR THE DECK FROM THE HYDRO-DEMOLITION REMOVAL AND HAND REMOVAL. FOR THE FIELD MEASUREMENT OF THIS ITEM, THE CONCRETE REQUIRED FOR THE DECK OVERLAY (\_\_\_\_\_ CUBIC YARDS) WILL BE DEDUCTED FROM THE TOTAL CONCRETE VOLUME PLACED DURING THE OVERLAY OPERATION. THE \_\_\_\_\_ CUBIC YARD QUANTITY WAS DETERMINED USING A 24 INCH OVERLAY THICKNESS (13 INCH NOMINAL PLUS 1/ INCH ALLOWABLE VARIATION, IT IS ASSUMED THE OVERLAY OPERATION WILL PLACE ALL DECK CONCRETE (EXCLUDING AREAS OF CLASS B REPAIR) IN ONE OPERATION.

THE BID ITEM "DECK OVEBLAY (CLASS O PPC)" OR "DECK OVERLAY (CLASS HPC-O PCC )" SHALL INCLUDE THE COST OF THE \_\_\_\_\_ CUBIC YARDS OF CONCRETE MATERIAL NOTED AS A DEDUCTION WHEN CALCULATING THE "CONCRETE REPAIR, REPLACE VARIABLE DEPTH CONCRETE" PAY QUANTITY.

THE BID ITEM "REMOVALS, CLASS A" SHALL INCLUDE COST OF LABOR AND EQUIPMENT REGUIRED TO REMOVE UNSOUND CONCRETE AND UNBONDED CONCRETE AROUND EXPOSED REINFORCING BARS AFTER HYDRODEMOLITION, REMOVALS WILL INVOLVE HAND CHIPPING TOOLS AND BE PAID FOR BY CONTRACT UNIT PRICE PER SQ.YD.

THE ENGINEER WILL DETERMINE THE SQ.YD. OF "REMOVALS, CLASS A" BY MEANS OF SURFACE DIMENSIONS OF THE AREAS TO BE REMOVED TO THE NEAREST O.I SQ. YD.

CONCERNING ADJUSTMENT OF PRICE FOR OVERRUN OR UNDERRUN OF THE CONTRACT QUANTITY, "REMOVALS, CLASS A" WILL NOT BE CONSIDERED A MAJOR ITEM OF WORK.

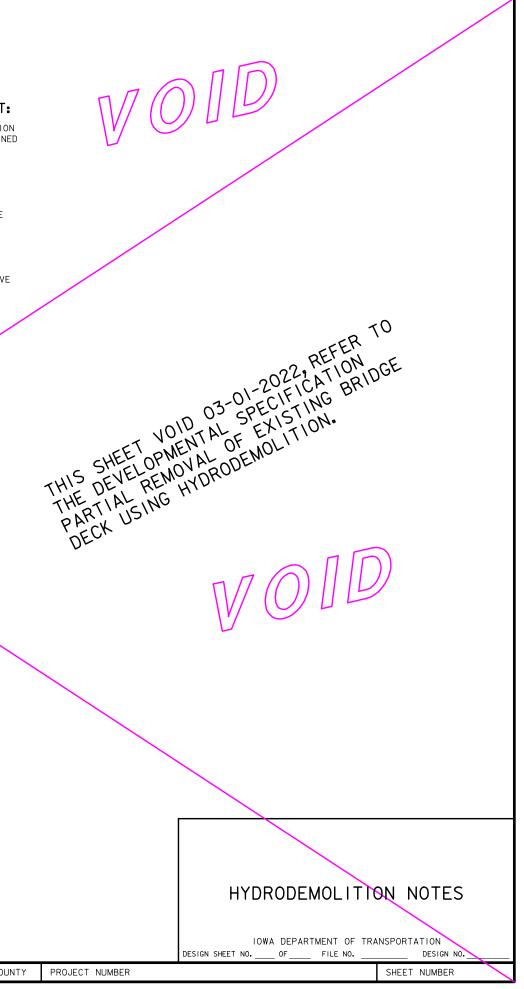
DEFECTS IN EMBEDDED REINFORCING STEEL DUE TO CORROSION, WHICH HAS REDUCED THE CROSS SECTIONAL AREA OF THE STEEL BY 25% OR GREATER, SHALL HAVE NEW REINFORCING STEEL OF THE SAME SIZE OR GREATER CROSS SECTIONAL AREA LAPPED TO EACH SIDE OF THE DAMAGED AREA. 2'-2" LAP LENGTHS SHALL BE USED. NEW REINFORCEMENT SHALL BE EPOXY COATED. NEW REINFORCEMENT SHALL BE PAID FOR AS AN EXTRA WORK ORDER.

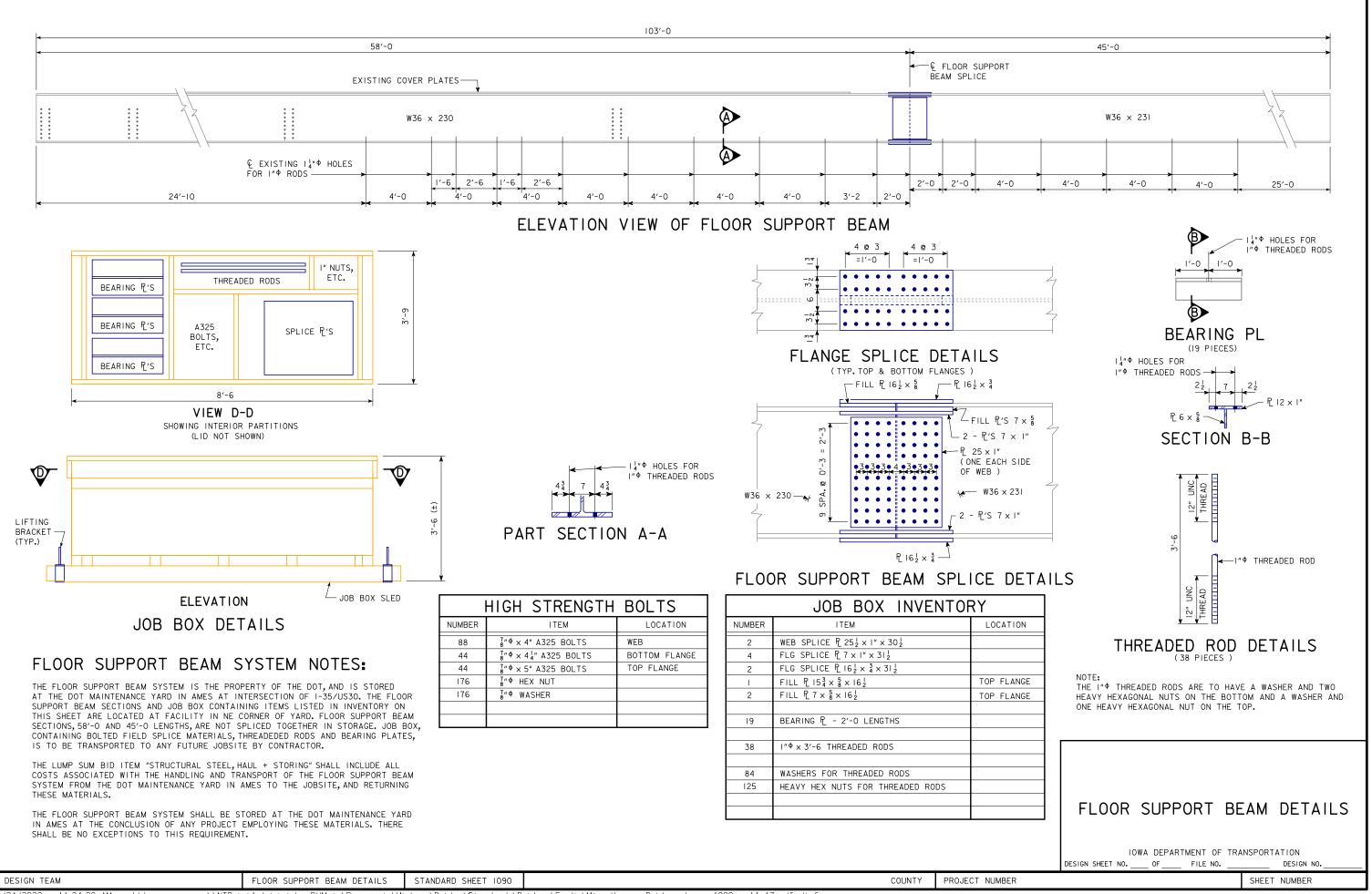
WHERE THE DECK IS UNSOUND FOR MORE THAN HALF OF ITS ORIGINAL DEPTH AS DETERMINED BY THE ENGINEER, THE CONCRETE SHALL BE REMOVED FULL DEPTH. FOR THESE AREAS OF FULL DEPTH REMOVAL (DESIGNED AS CLASS B REPAIR), THE WORK SHALL BE PAID FOR AS EXTRA WORK.



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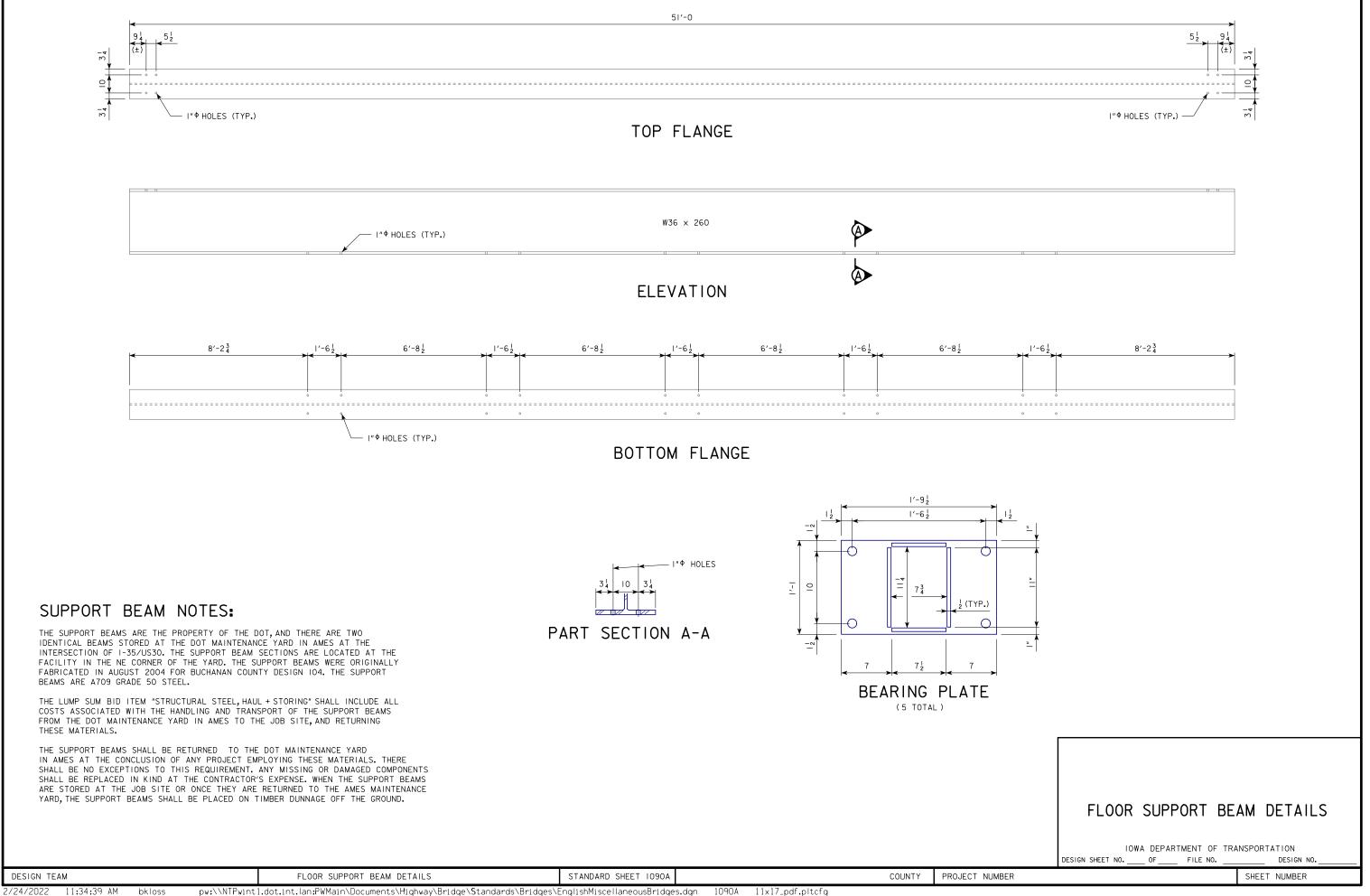
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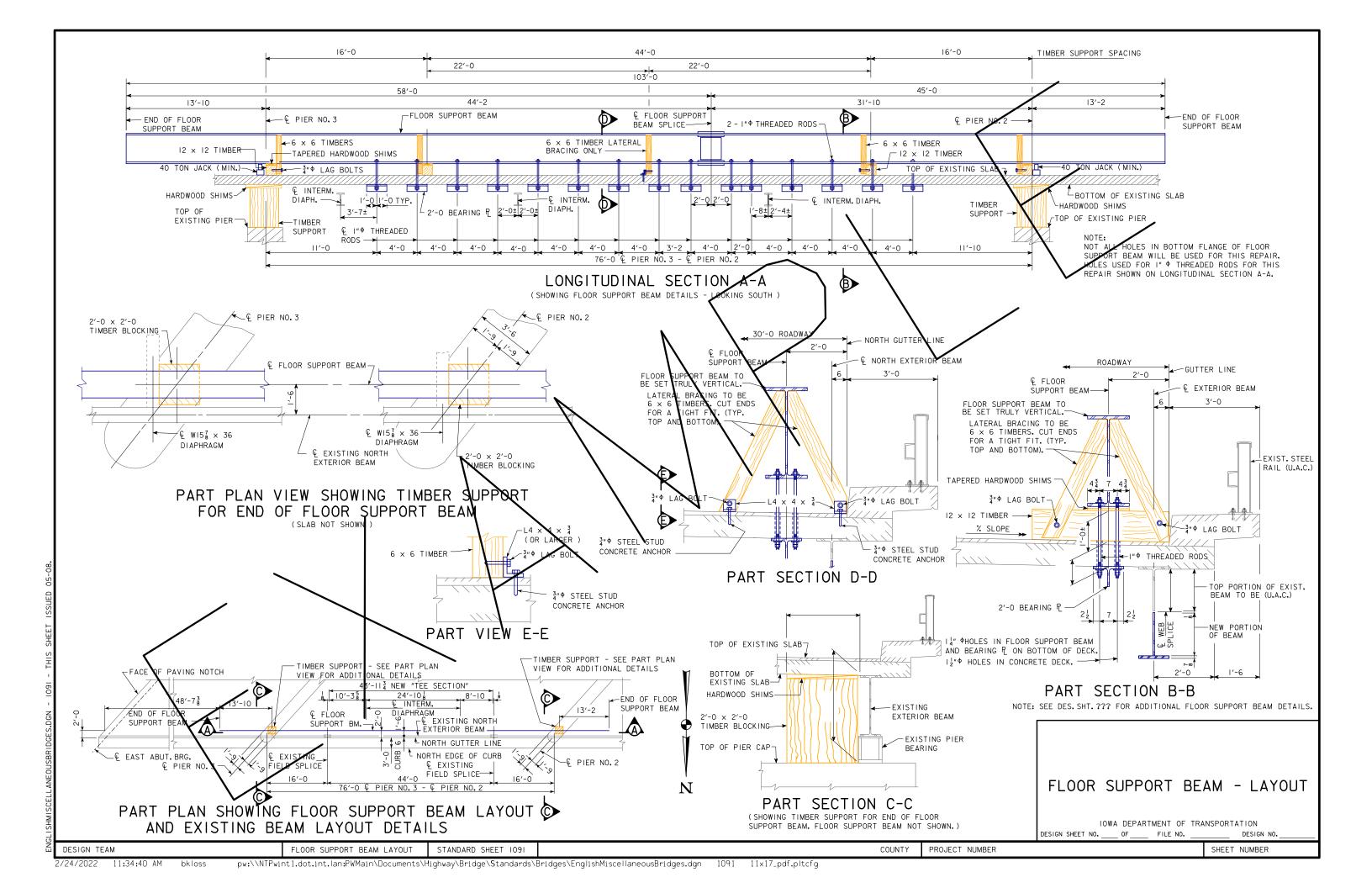
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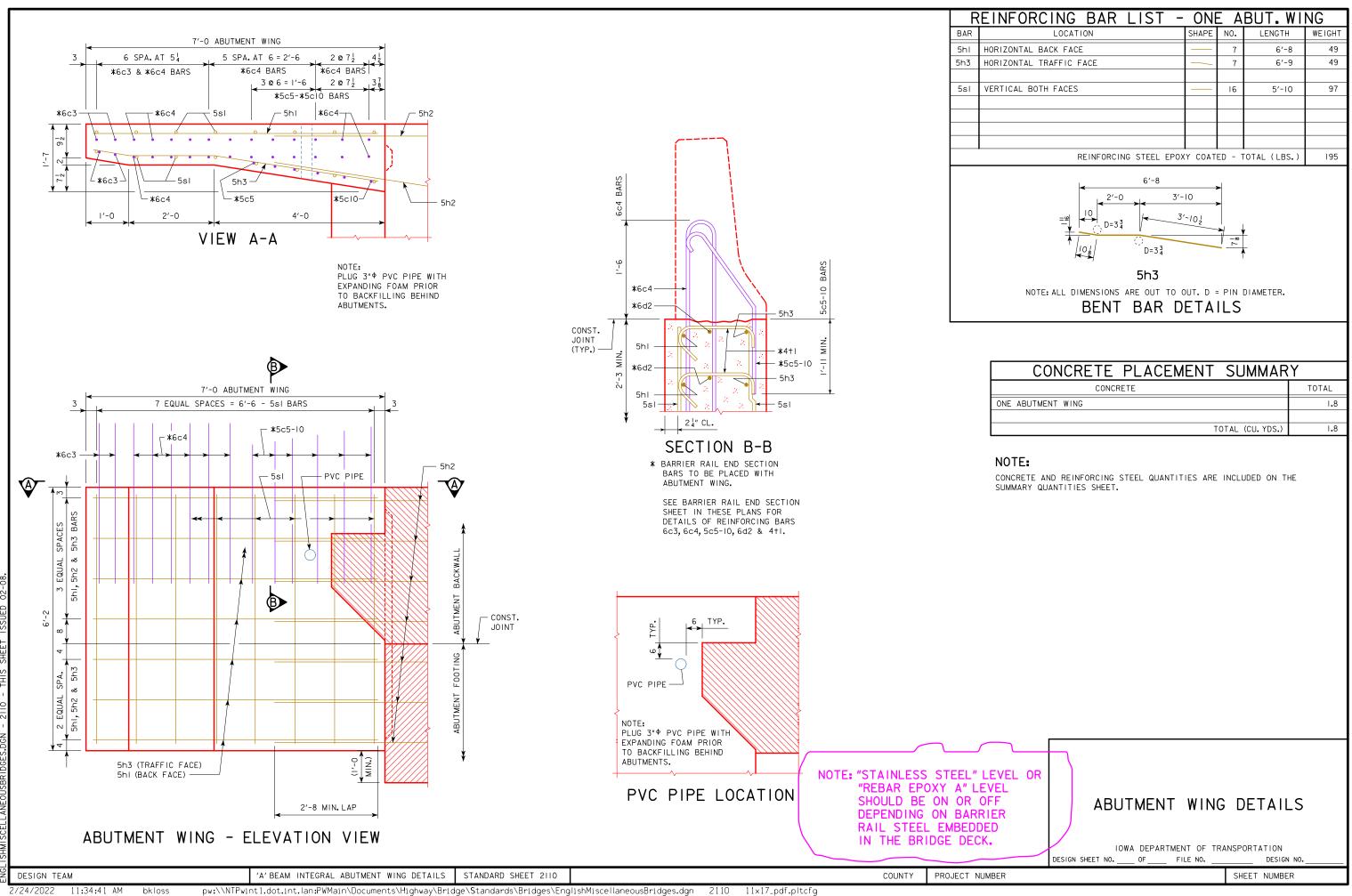
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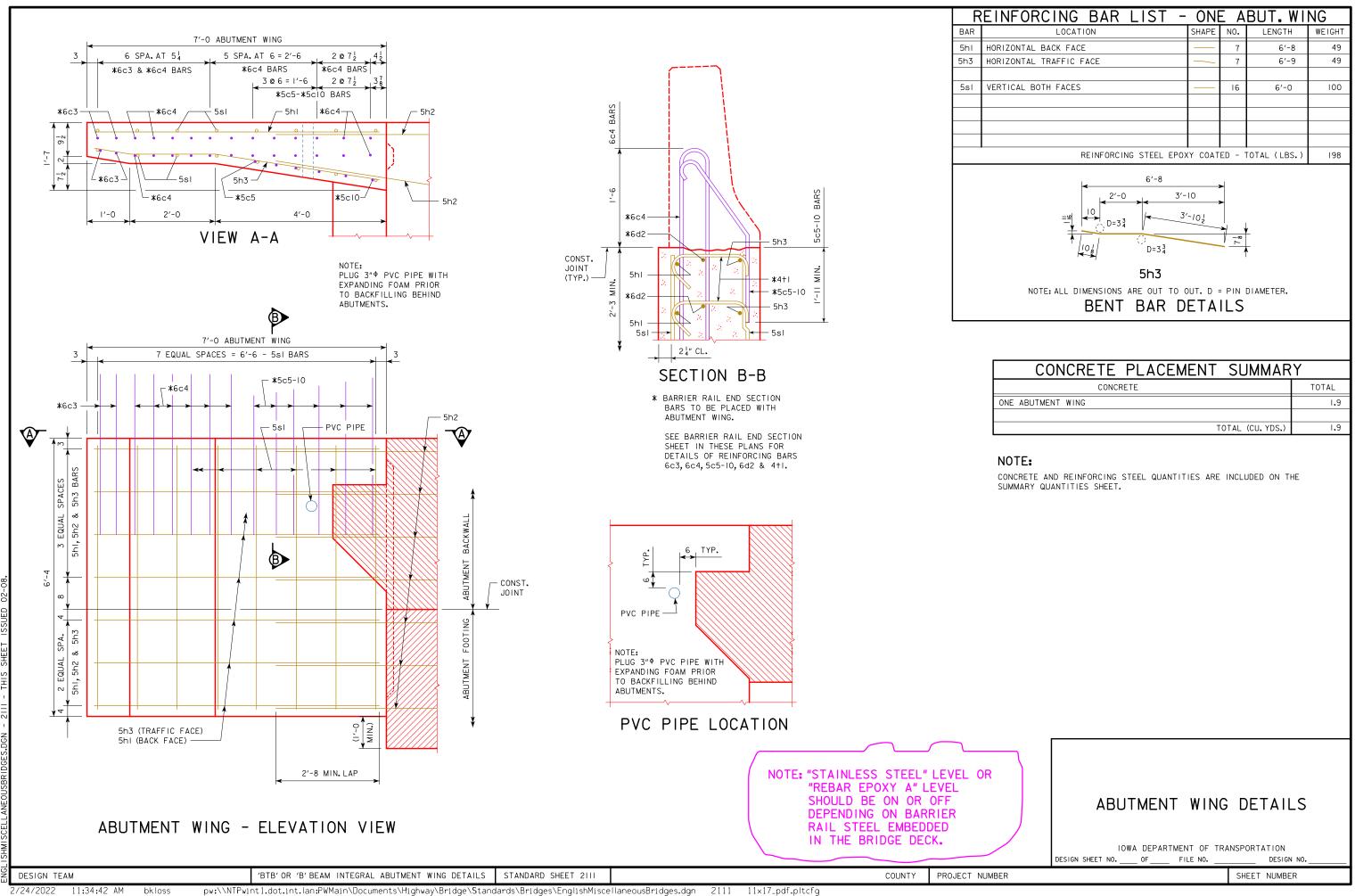


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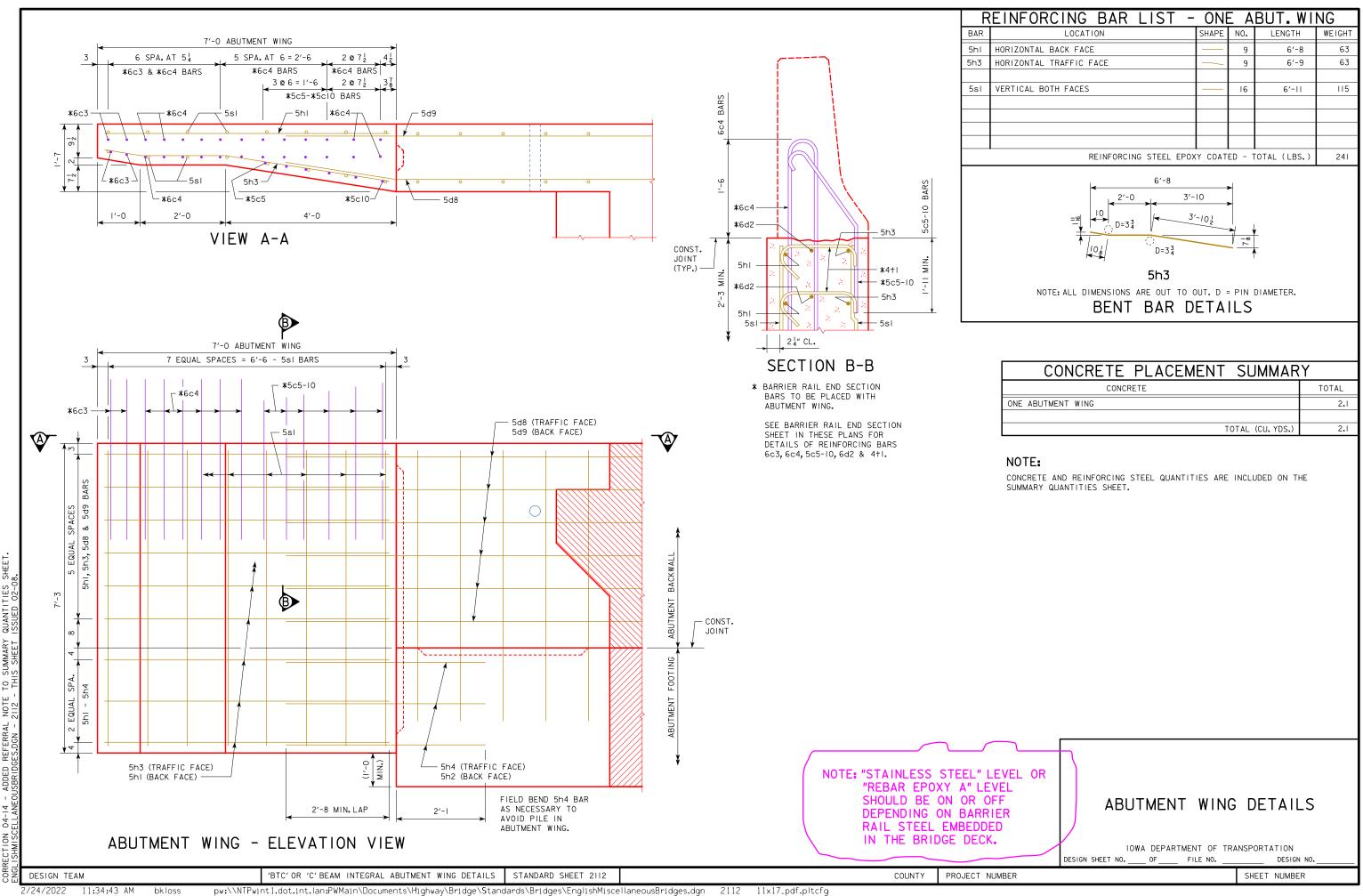




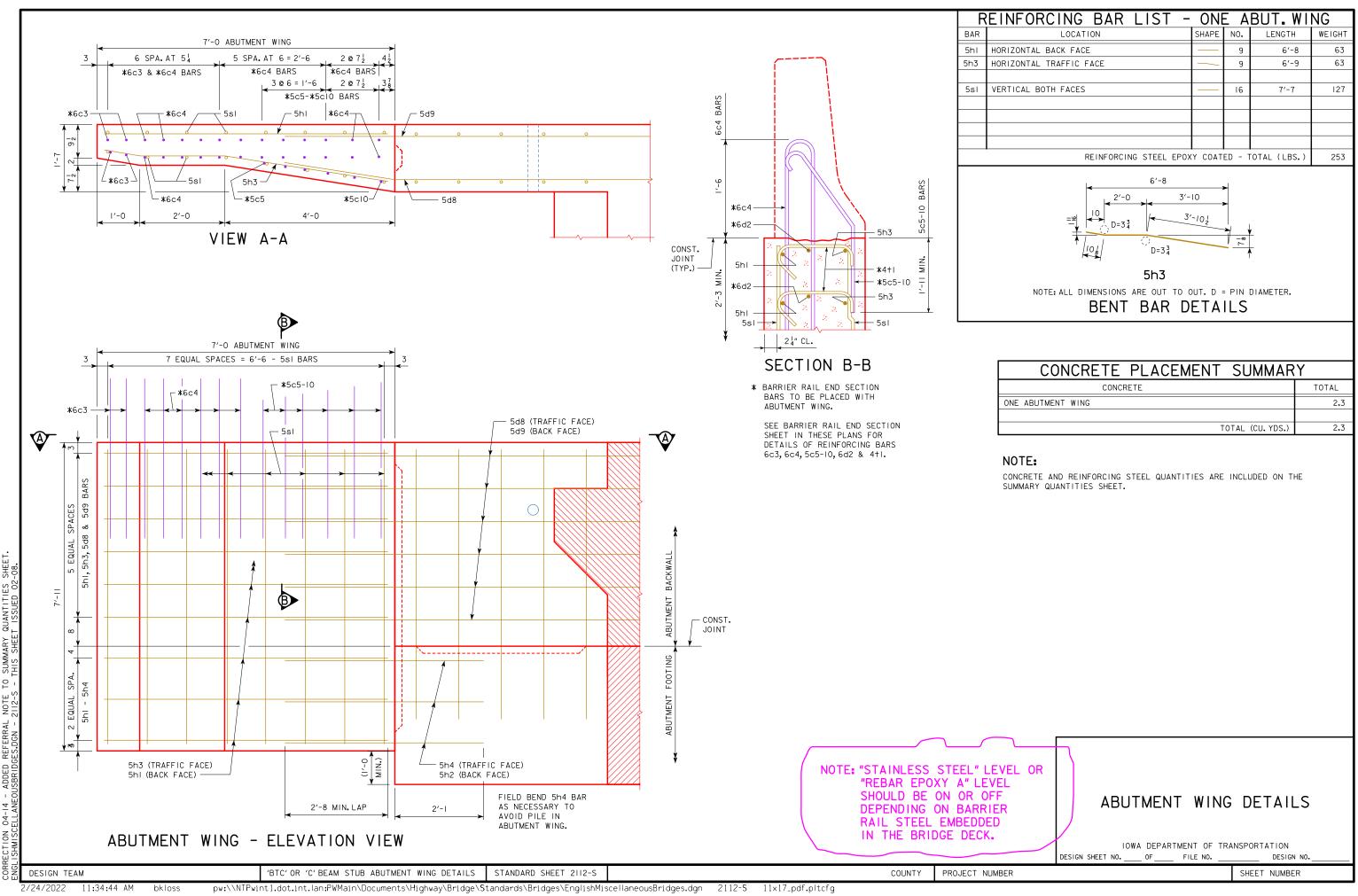
JORRECTION 04-14 - ADDED REFERRAL NOTE TO SUMMARY QUANTITIES SHEET. SNGLISHMISCELLANEOUSBRIDGES,DGN - Z110 - THIS SHEET ISSUED 02-08.



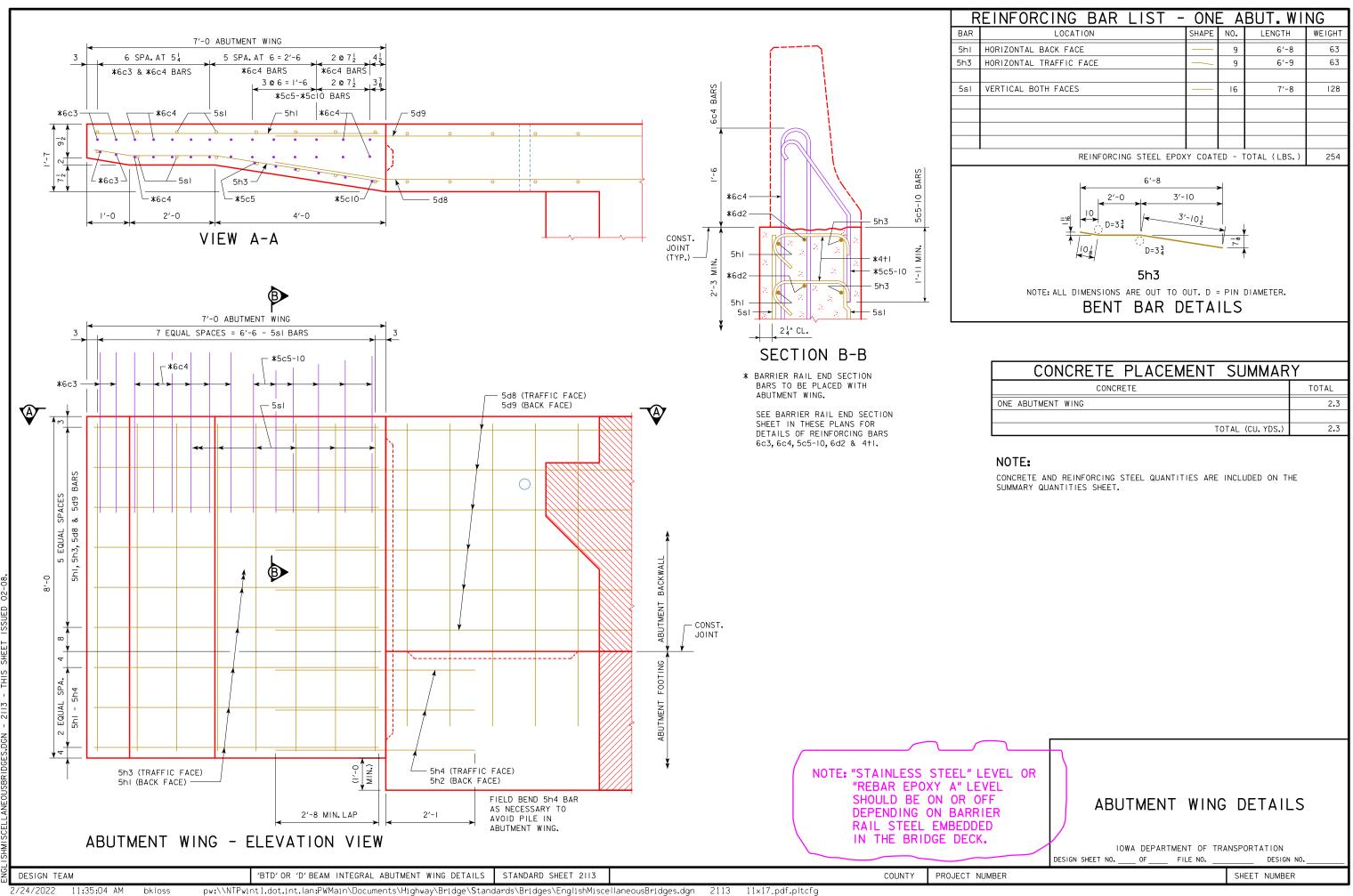
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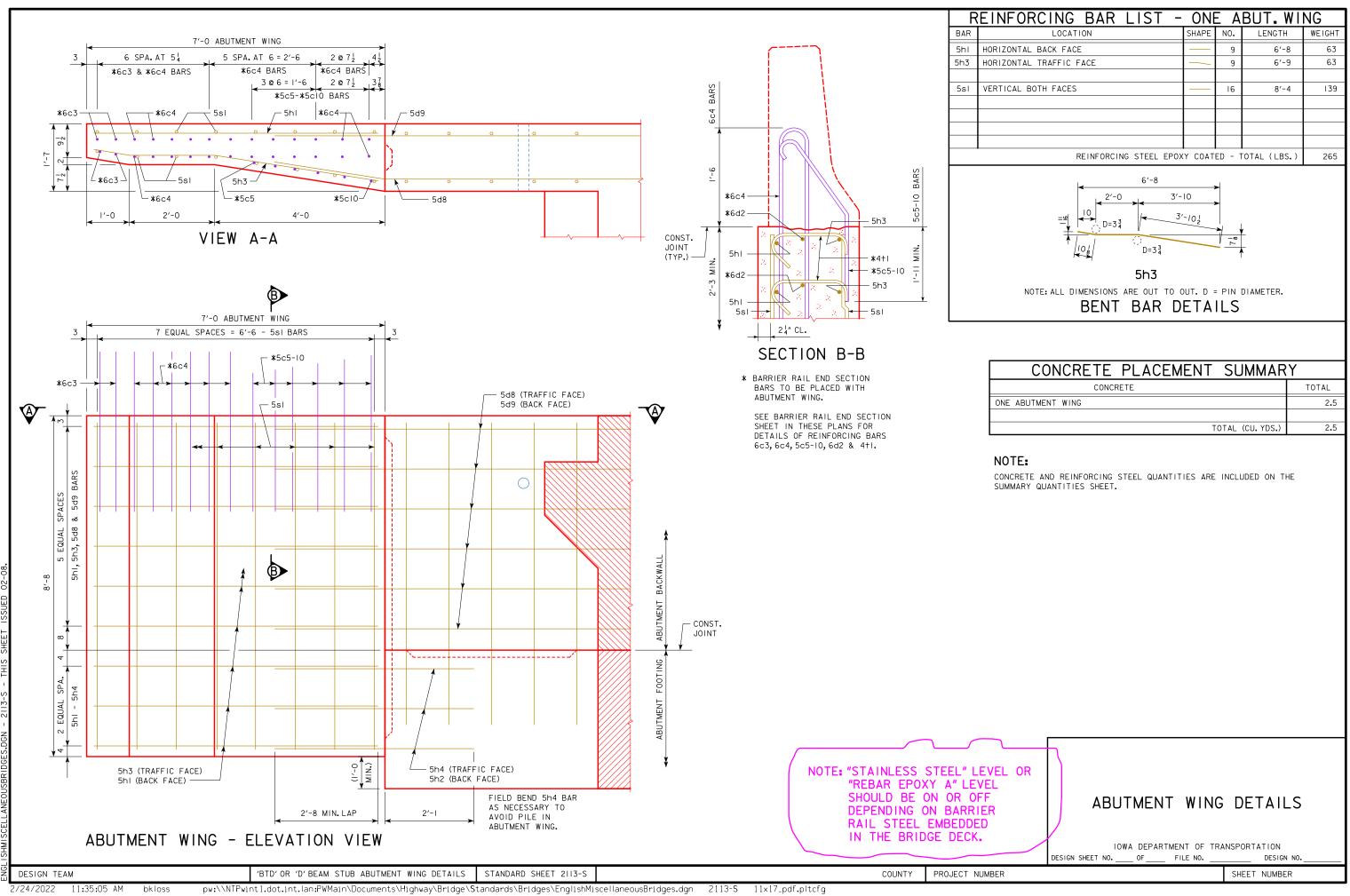
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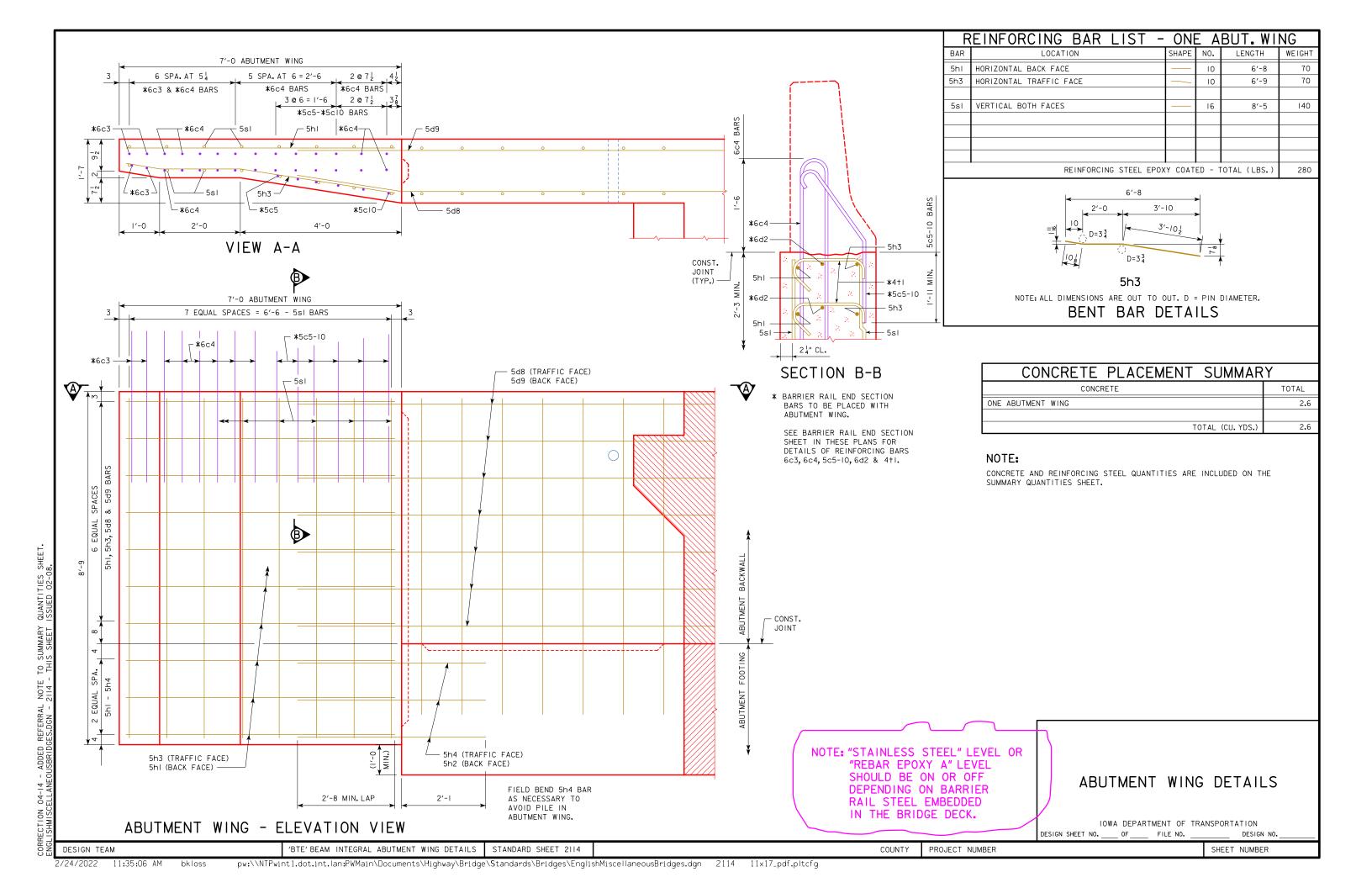
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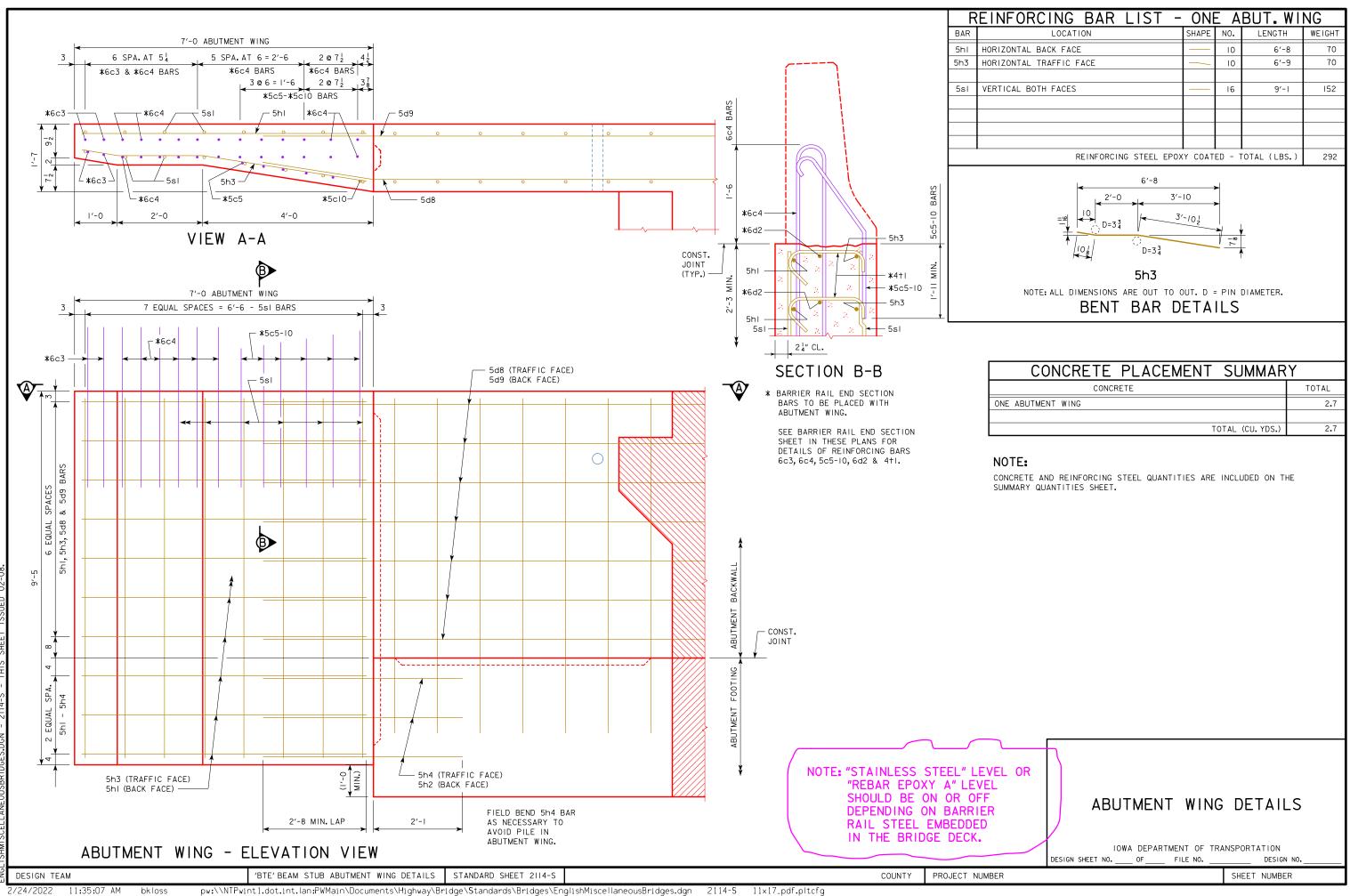


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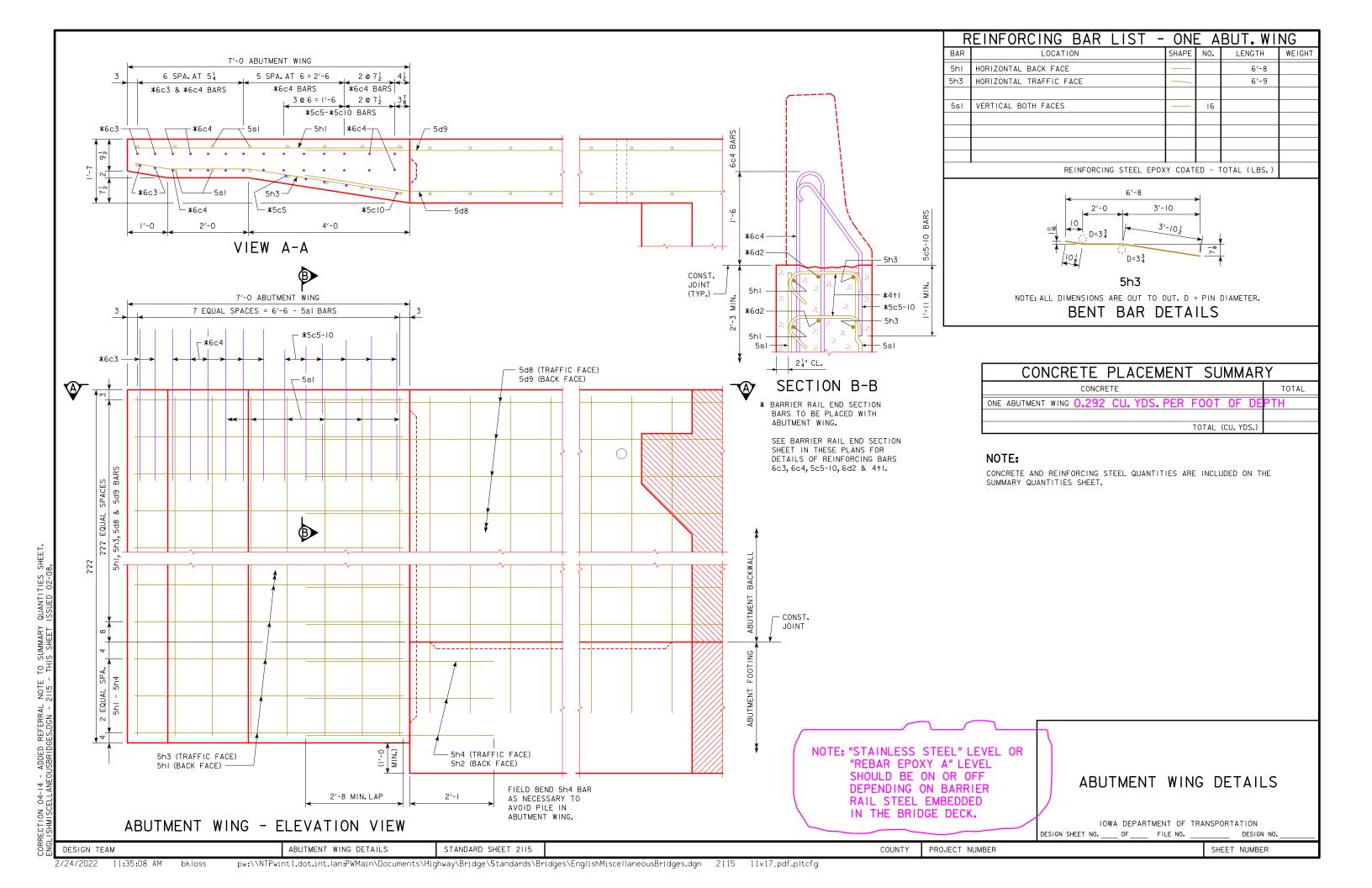
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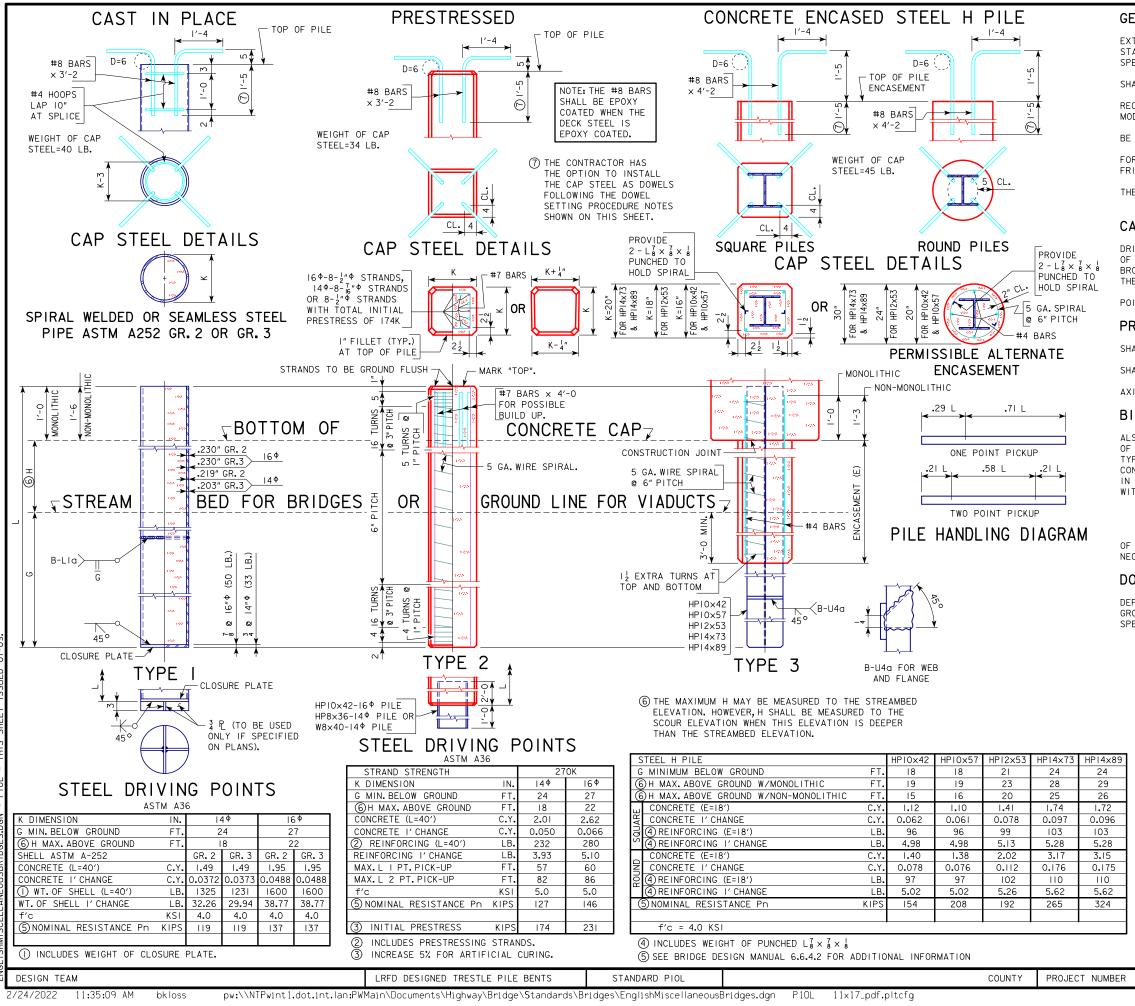




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REVISED 03-2022; UPDATED SPIRAL REQUIREMENTS TO ASTM A1054 GRADE 70 (WAS ASTM PROFISED AND A DATA AND A DATA

A82).

### GENERAL NOTES:

EXCEPT AS NOTED ELSEWHERE, MATERIAL, CONSTRUCTION, DRIVING AND EXTENSIONS OR BUILD UPS WHEN NECESSARY SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS OF THE IOWA D.O.T. AND CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS APPLICABLE.

CAP STEEL SHALL BE AS DETAILED ON THIS SHEET (D=PIN DIAMETER). IT SHALL BE USED IF PILE EMBEDMENT IS LESS THAN 1'-6.

"NOMINAL RESISTANCE Pn", "G", AND "H" AS GIVEN IN TABLES ARE RECOMMENDED DESIGN VALUES FOR ORDINARY CONDITIONS, BUT MAY BE MODIFIED FOR SPECIAL CONDITIONS ON ANY GIVEN JOB.

NOMINAL RESISTANCE Pn AND PILE SIZE REQUIRED SHALL IN ALL CASES BE AS SPECIFIED ON THE PLANS.

NOMINAL RESISTANCE Pn SHOWN ARE FOR FRICTION RESISTANCE EXCEPT FOR TYPE 3 PILING WHERE THE RESISTANCE VALUES SHOWN COULD BE EITHER FRICTION OR POINT RESISTANCE.

COST OF ALL DRIVING POINTS AND CAP STEEL IS TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT FOR PILING.

WIRE SPIRAL SHALL CONFORM TO ASTM A1064 GRADE 70.

### CAST IN PLACE PILE NOTES:

SHELL THICKNESSES SHOWN ARE MINIMUM REQUIREMENTS. THE METHOD OF DRIVING STEEL SHELL PILES SHALL BE ADAPTED TO THE TYPE AND THICKNESS OF SHELL SPECIFIED. ANY SHELLS WHICH HAVE BEEN IMPROPERLY DRIVEN, BROKEN OR ARE OTHERWISE DEFECTIVE SHALL BE REMOVED AND REPLACED BY THE BRIDGE CONTRACTOR.

ALL CAST IN PLACE PILES SHALL HAVE A CLOSURE PLATE. DRIVING POINTS SHALL BE USED IF SPECIFIED ON THE PLANS.

### PRESTRESSED PILE NOTES:

EXCEPT AS OTHERWISE NOTED ALL EXPOSED CORNERS 90° OR SHARPER SHALL BE FILLETED  $\frac{3}{4}".$ 

DRIVING POINTS FOR PRESTRESSED PILES, IF CALLED FOR ON THE PLANS, SHALL BE AS DETAILED.

HEADS OF PRESTRESSED PILES TO BE FINISHED SMOOTH AND NORMAL TO AXIS OF PILE.

### **BIDDING NOTES:**

THE PLANS SHALL DESIGNATE THE SIZE OF PILE TO BE USED. THEY SHALL ALSO SPECIFY THE TYPE, EITHER TYPE I, TYPE 2, OR TYPE 3. IF THE OPTION OF TYPE I OR 2 IS GIVEN ON THE PLANS, THE CONTRACTOR SHALL CHOOSE THE TYPE TO BE USED. IF TYPE 3 IS SPECIFIED, TYPE 3 SHALL BE USED, BUT THE CONTRACTOR MAY CHOOSE THE SHAPE OF THE ENCASEMENT. IT SHOULD BE KEPT IN MIND THAT FOR A GIVEN SIZE AND RESISTANCE VALUE, LENGTH MAY VARY WITH THE SHAPE (SQUARE OR ROUND).

PILES SHALL BE BID DESIGNATING THE SIZE, TYPE AND LENGTH.

TYPE I PILING WILL BE BID PER LINEAR FOOT OF PILE.

TYPE 2 PILING WILL BE BID PER LINEAR FOOT OF PILE.

TYPE 3 PILING WILL BE BID PER LINEAR FOOT OF PILE AND LINEAR FOOT OF ENCASEMENT. PRICE BID FOR ENCASEMENT SHALL BE FULL PAYMENT FOR NECESSARY EXCAVATION AND FOR FURNISHING AND PLACING ALL MATERIAL.

### DOWEL SETTING PROCEDURE:

IF CAP STEEL IS REQUIRED FOR THE PRESTRESSED PILES THE #8 DEFORMED BARS ARE TO BE SET AS DOWELS INTO THE PILES WITH POLYMER GROUT IN ACCORDANCE WITH ARTICLE 2301.03, E, OF THE STANDARD SPECIFICATIONS OR BY THE FOLLOWING PROCEDURE.

-A.) DRILL HOLE APPROXIMATELY TWICE THE DIAMETER OF THE DOWEL BAR AND TO THE DEPTH INDICATED.

AND TO THE DEFTH INFIGURED. -B.) FILL HOLE WITH WATER AND ALLOW TO STAND LONG ENOUGH TO THOROUGHLY SATURATE THE SURROUNDING CONCRETE (ABOUT FOUR HOURS). -C.) BLOW OUT ALL FREE WATER AND FILL HOLE 2/3 FULL OF MORTAR. -D.) INSERT DOWEL BY DRIVING, IF NECESSARY, AND MANIPULATE OR TAP WITH A HAMMER TO CONSOLIDATE MORTAR AND SECURE COMPLETE EMBEDMENT.

-E.) ADD MORE MORTAR, IF NECESSARY, TO FILL HOLE.

-F.) MORTAR SHALL CONSIST OF EQUAL PARTS PORTLAND CEMENT AND SAND WITH JUST ENOUGH WATER TO MAKE A WORKABLE MIX.



STANDARD DESIGN

CONCRETE AND STEEL PILES CAST IN PLACE, PRESTRESSED AND ENCASED FOR USE IN LRFD TRESTLE PILE BENTS - PIOL IOWA DEPARTMENT OF TRANSPORTATION DESIGN SHEET NO. \_\_\_\_\_ OF \_\_\_\_\_ FILE NO. \_\_\_\_\_ DESIGN NO. \_\_\_\_\_\_ SHEET NUMBER