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POWERING THE NEW ENGINEER TO TRANSFORM THE FUTURE

ConcreteWorks Software For Iowa Use

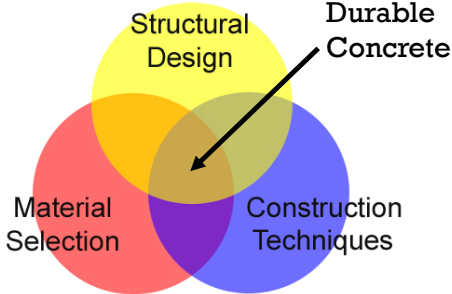


Thermal Cracking



Picture courtesy of J.C. Liu, TxDOT

Achieving Durable Concrete




Structural Design
 Material Selection
 Construction Techniques
 Durable Concrete

Concrete Mixtures

Infinite number of combinations of:

- Aggregate type & quantity
- Type & amount of cement
- SCM use
- Admixtures



Why do we model temperature and stress?

- Quality concrete
- Multi-variate problem –
 - rules of thumb can be inaccurate or unconservative
- Preplanning
 - Engineer – Best during design
 - Contractor – Best during bidding
- Required by specification



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Self-Generated Restraint

- Why is the bread surface cut before baking?

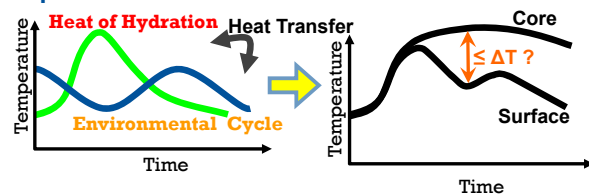


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Temperature Prediction



Material Factors
 Cementitious Types/Composition
 Cementitious Fineness
 Cementitious Content
 Chemical Admixtures
 w/cm
 Fresh Temperature
 Aggregate Types

Construction Factors
 Element Geometry/Size
 Insulation
 Form Properties
 Curing Methods
 Surface Color
 Cooling Pipes

Environment
 Air Temperature
 Wind Speed
 Relative Humidity
 Cloud Cover
 Solar Radiation
 Soil
 Water Submersion

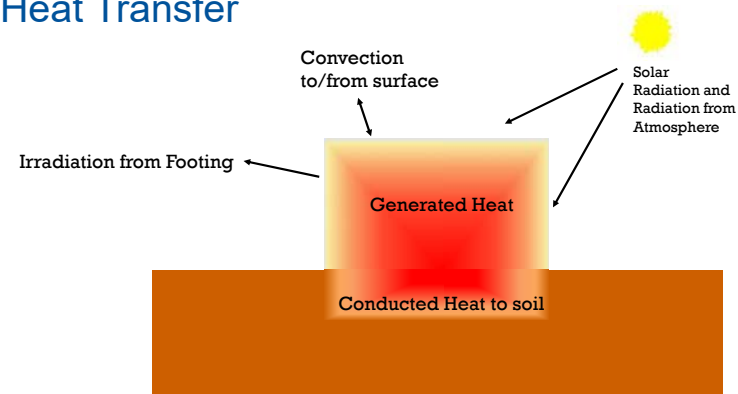


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Heat Transfer

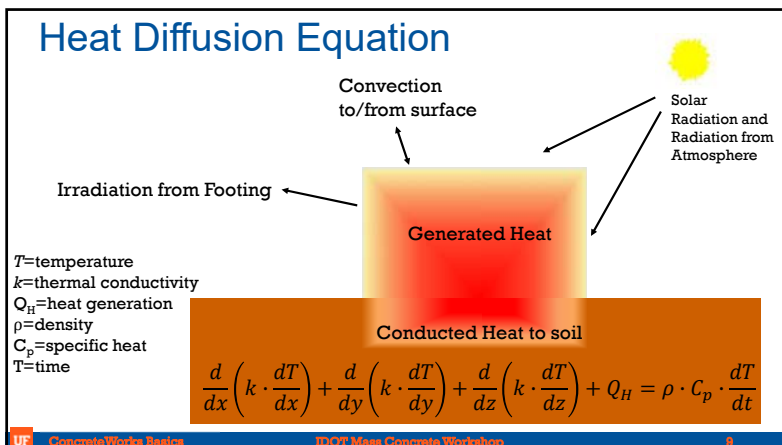


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Heat Diffusion Equation



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Temperature Prediction Methods

Increasing Complexity

- Estimate Max Temperature
- Schmidt Method
- ConcreteWorks
- Proprietary software (Finite Element Analysis)

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Heat Evolved at Each Time Step

(Schindler, 2002)

$$Q_h(t) = H_u \cdot W_c \cdot \left(\frac{\tau}{t_e} \right)^\beta \cdot \left(\frac{\beta}{t_e} \right) \cdot \alpha(t_e) \cdot \exp \left(\frac{E_a}{R} \left(\frac{1}{T_r} - \frac{1}{T_c} \right) \right)$$

From Literature (points to H_u)

From Semi-Adiabatic Calorimetry (points to τ , β , $\alpha(t_e)$)

From Isothermal Calorimetry (points to E_a)

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Role of Temperature in Hydration



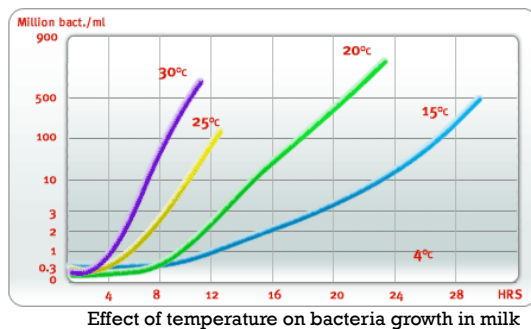
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Temperature Sensitivity Example



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Arrhenius Equation

Reaction Rate (S. Arrhenius, 1889)

Where

- k = rate of reaction
- A = constant ($\neq 0$)
- R = Universal gas constant (8.314)
- T = reference temperature
- E_a = Activation Energy

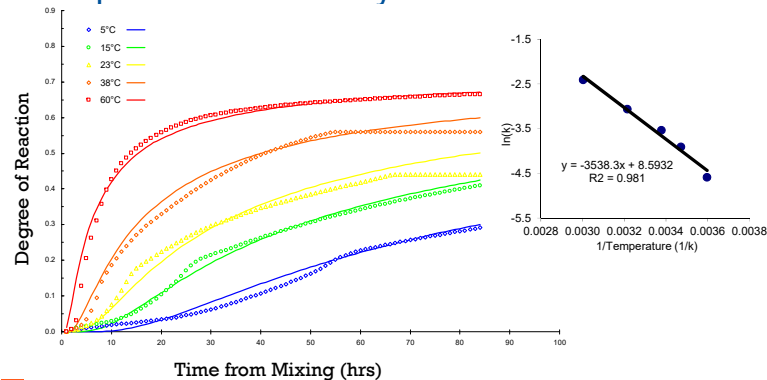
$$\ln k = \ln A - \frac{E_a}{RT}$$

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Temperature Sensitivity of Cement Reaction

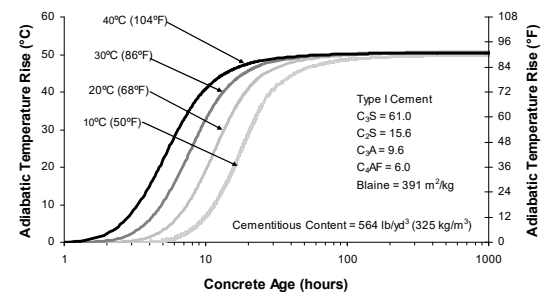


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Effects of Curing Temperature on Adiabatic Temperature Rise



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Heat Evolved at Each Time Step

(Schindler, 2002)

$$Q_h(t) = H_u \cdot W_c \cdot \left(\frac{\tau}{t_e} \right)^\beta \cdot \left(\frac{\beta}{t_e} \right) \cdot \alpha(t_e) \cdot \exp \left(\frac{E_a}{K} \left(\frac{1}{T_r} - \frac{1}{T_c} \right) \right)$$

From Literature (points to H_u)

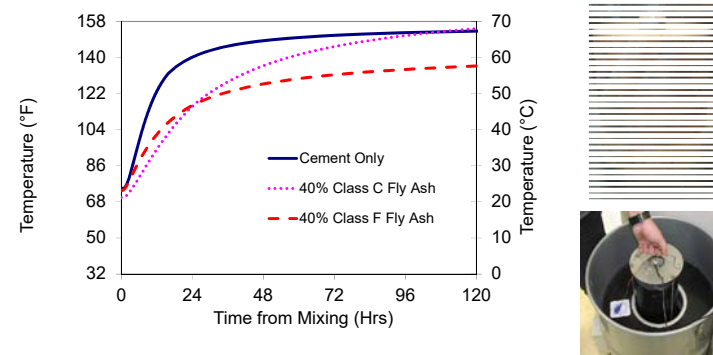
From Semi-Adiabatic Calorimetry (points to τ and β)

From Isothermal Calorimetry (points to $\alpha(t_e)$)

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Adiabatic or Semi-Adiabatic Calorimetry



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α –Degree of Hydration

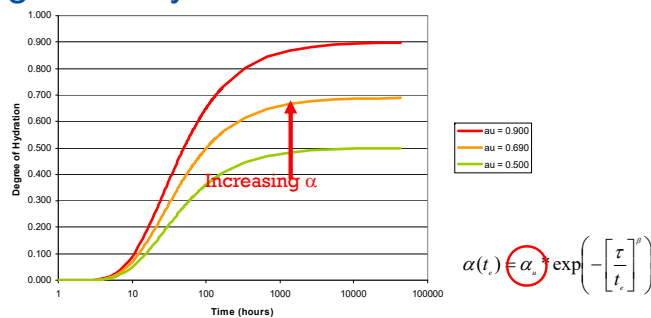


Figure Courtesy of Jonathan Poole

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τ –Time Parameter

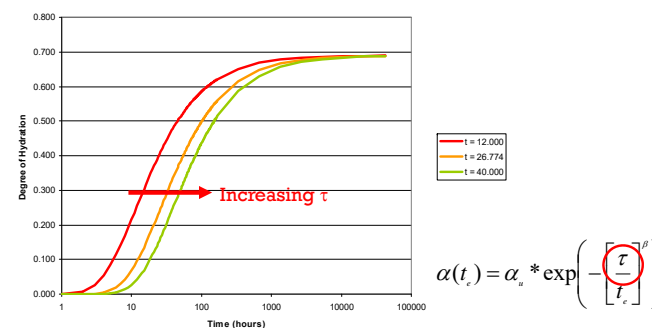
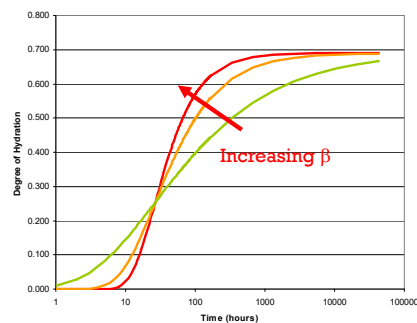


Figure Courtesy of Jonathan Poole

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β – Slope Parameter



$$\alpha(t_e) = \alpha_{\infty} * \exp\left(-\left[\frac{\tau}{t_e}\right]^{\beta}\right)$$

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Model for Hydration Built From:

- E_a Trends – From 116x5 Isothermal tests
- α_{∞} , τ , β , H_u Trends – From Semi-Adiabatic Data – 204 Semi-Adiabatic Tests
- Validation performed using data from Schidler (2005), Ghe Li (2006), and field sites – 58 Semi-Adiabatic Tests
- Variability of test methods is quantified – 63 Semi-Adiabatic Tests.
- A brief overview of the trends seen in the study is shown next.

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Variable	Range of Tests	Effect on τ	Effect on β	Effect on α_{∞}
Fly Ash (%Replacement)	15-55%			
Fly Ash (CaO%)	0.7-28.9% CaO			Varies
GGBF slag	30-70%	Large	Small	Varies
Silica Fume	5-10%	None	None	Small




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Variable	Range of Tests	Effect on τ	Effect on β	Effect on α_{∞}
LRWR	0.22-0.29%	Varies	Small	Varies
WRRET	0.18-0.53%	Large	Large	Large
MRWR	0.34-0.74%	Large	Small	Varies
HRWR	0.78-1.25%	None	Small	Large
PCHRWR	0.27-0.68%	None	Small	Large
ACCL	0.74-2.23%	Small	None	Varies
AEA	0.04-0.09%	None	None	None

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Variable	Range of Tests	Effect on τ	Effect on β	Effect on α_u
Increasing w/c	0.32-0.68	None	None	Large 
Placement Temp	15-38 °C (50-100 °F)	None	None	None
Increase Cement Fineness	350-540 m ² /kg	Small 	Small 	Varies

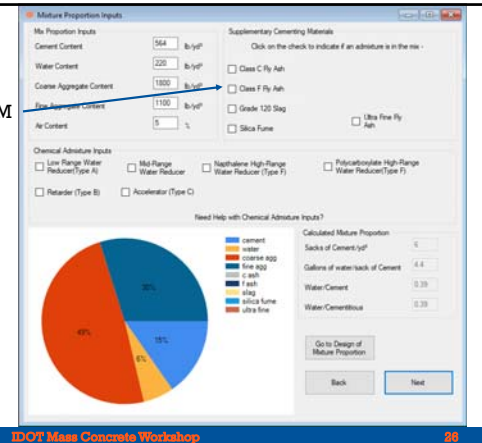
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Proportions

Click checkboxes to use an SCM



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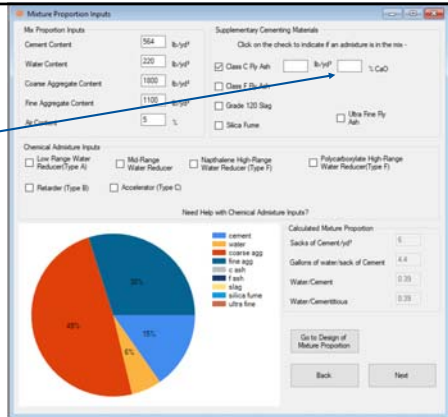
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Proportions

Don't forget to enter CaO content of fly ashes

Check when admixtures used.

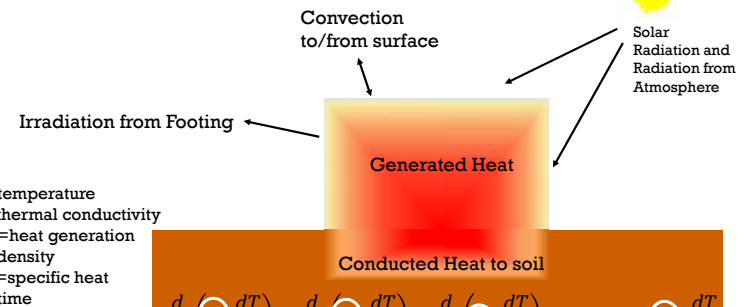


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Heat Diffusion Equation



$$\frac{d}{dx} \left(k \frac{dT}{dx} \right) + \frac{d}{dy} \left(k \frac{dT}{dy} \right) + \frac{d}{dz} \left(k \frac{dT}{dz} \right) + Q_H = \rho \cdot C_p \cdot \frac{dT}{dt}$$

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Aggregates

Material Properties Input Screen

- Concrete thermal properties based on aggregate combination selected
- Can input your own thermal properties by checking box (for example, to use lightweight aggregate)

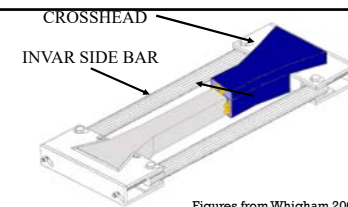
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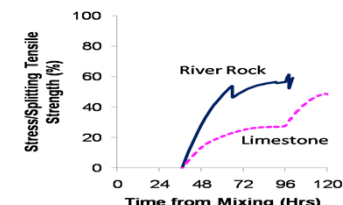
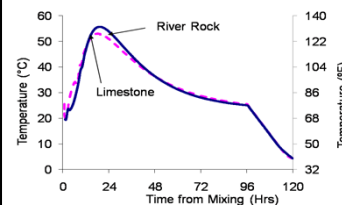
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Aggregate Type

Coarse Aggregate Concrete CTE
 River Rock 10.5 $\mu\epsilon/^{\circ}\text{C}$
 Limestone 6.6 $\mu\epsilon/^{\circ}\text{C}$



Figures from Whigham 2005



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Aggregates

$$\alpha_{\text{calc}} = \frac{\alpha_{ca} \cdot V_{ca} + \alpha_{fa} \cdot V_{fa} + \alpha_p \cdot V_p}{V_{ca} + V_{fa} + V_p}$$

Material	Coefficient of Thermal Expansion values used in ConcreteWorks ($\mu\epsilon/^{\circ}\text{C}$)	Coefficient of Thermal Expansion from Emanuel and Hulsey, 1977 ($\mu\epsilon/^{\circ}\text{C}$)
Hardened Cement Paste	10.8	10.8
Limestone Aggregate	3.5	3.5 - 6
Siliceous River Gravel and Sand	11	11 - 12.5
Granite Aggregate	7.5	6.5 - 8.5
Dolomitic Limestone Aggregate	7	7 - 10

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Construction Inputs

Default is use air temperature at placement

Options: steel, wood, and insulated steel formwork

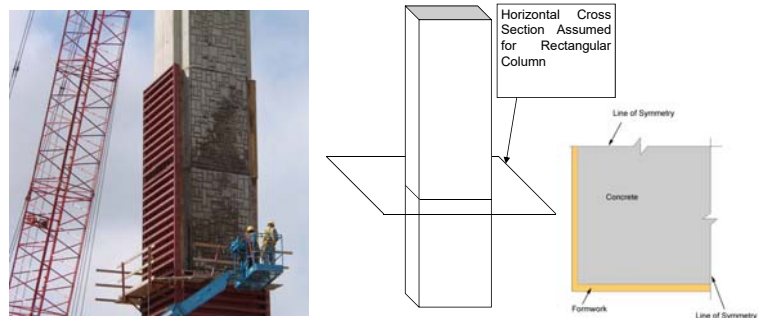
*If insulated steel forms used, form insulation value appears

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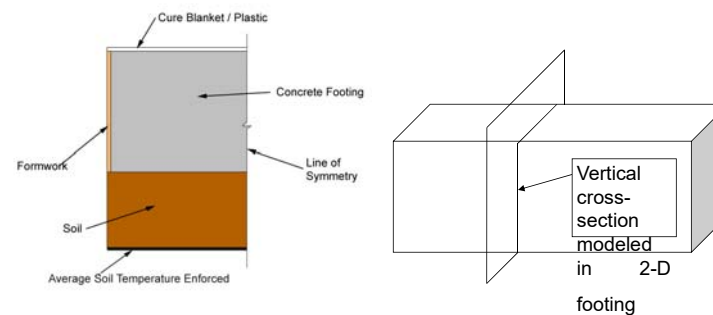
ConcreteWorks Geometry: Columns



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ConcreteWorks Geometry: Footings



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Foundation Thermal Properties

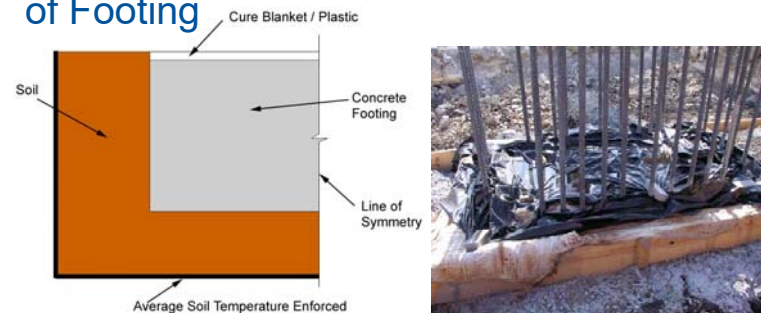
Subbase Material	Density (kg/m ³)	Thermal Conductivity (W/m/K)	Specific Heat (J/kg/K)	Reference
Clay	1460	1.3	880	Incropera and Dewitt, 2002
Granite	2630	2.79	775	
Limestone	2320	2.15	810	
Marble	2680	2.8	830	
Quartzite	2640	5.38	1105	
Sandstone	2150	2.9	745	
Sand	1515	0.27	800	
Top Soil	2050	0.52	1840	
Concrete*	-	-	-	

*Concrete is assumed to have the same thermal properties of the concrete used on the footing, with a degree of hydration equal to 0.6.

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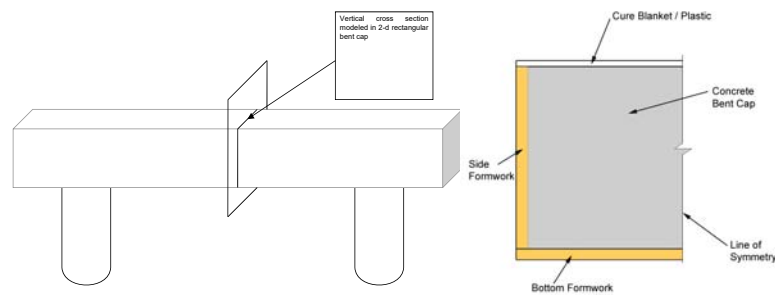
ConcreteWorks Geometry: Soil on Sides of Footing



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Bent Cap

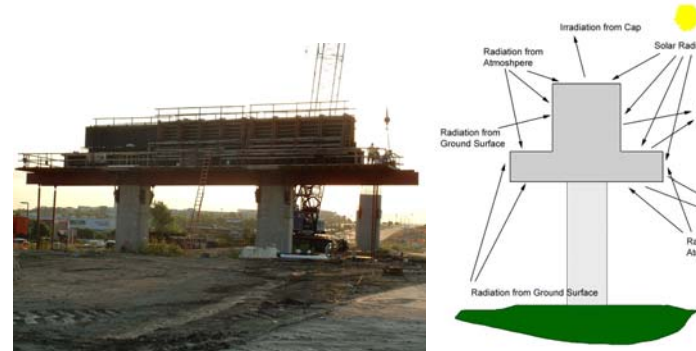


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T-shaped Bent Caps

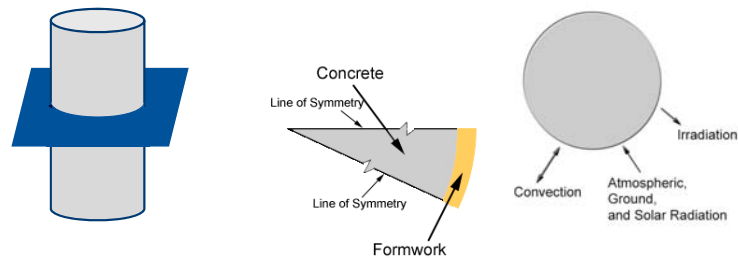


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Circular Column/ Drilled Shaft



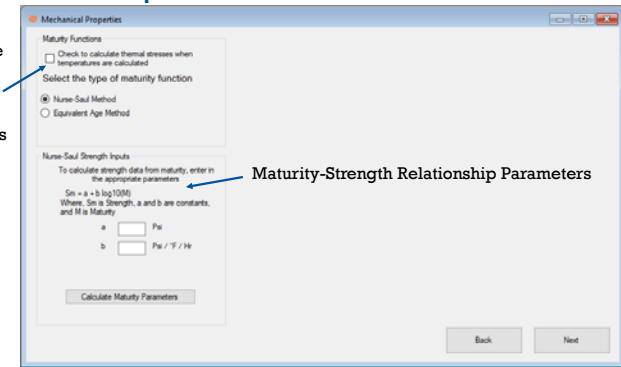
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Mechanical Properties

Check to calculate stresses – can be slow, not recommended for more than 4-5 days of analysis



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Environment Inputs (Weather)

Weather data based on 30 year average values

Click here to manually change values

Day	Max	Min
1	45.5	17.9
2	49.6	36.9
3	52.7	38.3
4	53.6	37.4
5	49.3	37.2
6	48.6	36.9
7	47.7	34.3
8	40	34.7

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Service Life Modeling

Used for corrosion service life model inputs

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Thermocouple Points

Allows the user to select where sensors would be placed to give realistic estimate of values they would measure

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Acknowledgements

- TxDOT (original software development) & IDOT for providing funding for this work



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Problems to Work Out After Lunch

- Design a concrete control plan for a 8 ft by 10 ft column placed in Ames in August 2020 to meet IDOT standards.
- Next, determine any changes that would be needed to place the same column in January, 2021 in Ames.

*Pay attention to the difference it makes where the temperature sensor is placed



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Problems to Work Out After Lunch

- Design a control plan for concrete footing that is 20 ft by 30 ft by 8 ft thick in Des Moines in March. Use limestone subbase. What difference do you get between 1D and 2D analysis?
- What about with a 10 ft wide footing – what difference do you get between a 1D and 2D analysis?



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Problems to Work Out After Lunch

- You want to place a concrete footing that is 30 ft by 20 ft by 6 ft thick. It is August in Des Moines, and you expect a storm after 2 days to lower the high temperature to 60° F and the low temperature on day 2 to 45° F. Design a system that will still meet IDOT standards.



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