

IM-235-2 (313) 8--13-77
 BRIDGE REPLACEMENT - STEEL GIRDER

POLK COUNTY

LETTING DATE
 10-25-2005

CONVENTIONAL SIGNS

- DIVIDED HIGHWAY
- PAVED ROAD
- BITUMINOUS ROAD
- GRAVEL ROAD
- EARTH ROAD
- INTERSTATE HIGHWAY
- UNITED STATES HIGHWAY
- STATE HIGHWAY
- COUNTY HIGHWAY
- RAILROAD
- PIPELINE
- AIRPORT
- HYDROLOGY
- BRIDGE
- STATE BOUNDARY
- COUNTY BOUNDARY
- CORPORATE LIMIT LINE
- TOWNSHIP LINE
- SECTION LINE

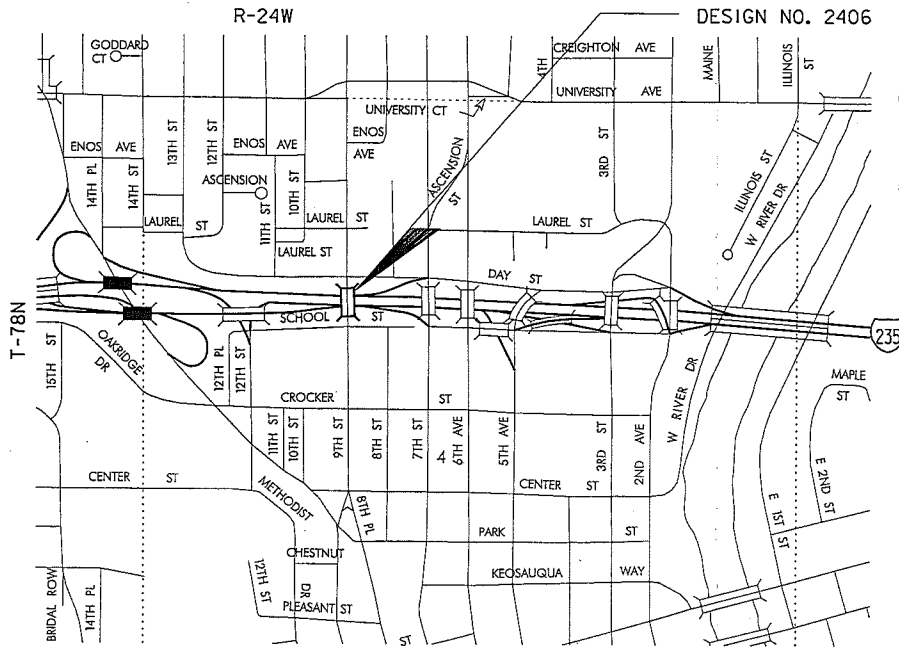


Iowa Department of Transportation
 Highway Division

PLANS OF PROPOSED IMPROVEMENTS ON THE
INTERSTATE ROAD SYSTEM
POLK COUNTY
 BRIDGE REPLACEMENT - STEEL GIRDER
 ON 9th ST. OVER
 I-235

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

VALUE ENGINEERING SAVES. REFER TO THE GENERAL NOTES IN THESE PLANS.



PART OF CITY OF DES MOINES

THESE PLANS WERE PREPARED FOR THE IOWA DEPARTMENT OF TRANSPORTATION BY: PARSONS

ALL WORKING DRAWINGS INCLUDING SHOP DRAWINGS AND FALSEWORK DRAWINGS WILL BE REVIEWED BY:

PARSONS
 10 SOUTH RIVERSIDE, SUITE 400
 CHICAGO, IL 60606

PROJECT DIRECTORY NAME: 77235180C96 - SECTION 5

DESIGN TEAM: METRIC IOWA DOT * OFFICE OF BRIDGES AND STRUCTURES FILE NO. 29552 POLK COUNTY PROJECT NUMBER IM-235-2 (313) 8--13-77 SHEET NUMBER 1

TOTAL SHEETS
PROJECT NUMBER
IM-235-2 (313) 8--13-77
R.O.W. PROJECT NUMBER
PROJECT IDENTIFICATION NUMBER
99-77-235-332-05

INDEX OF SHEETS	
NO.	DESCRIPTION
1	TITLE SHEET
1A	REVISION SHEET
2	BRIDGE ESTIMATE SHEET
2A	BRIDGE GENERAL NOTES
2-42	BRIDGE DESIGN NO. 2406
43	UTILITY ESTIMATE SHEET
43-44	UTILITY SHEETS
45-50	FENCE SHEETS
51	CIP WALL ESTIMATE SHEET
51-57	CIP WALL DESIGN NO. 2606

INDEX OF SEALS		
SHEET NO.	NAME	TYPE
1	ROBERT A. MAGLIOLA	STRUCTURAL DESIGN
5	KOLE C. BERG	GEOTECHNICAL DESIGN
45	WILLIAM D. TUCKER	STRUCTURAL DESIGN

REVISIONS	
1/22/07, SEE SHEET 1A	
4/25/07, SEE SHEET 1A	

DESIGN DATA URBAN (I-235)			
1996 AADT	48,500	V.P.D.	
2025 AADT	74,100	V.P.D.	
TRUCKS	8%		

STRUCTURAL DESIGN

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

Signature: *Robert A. Magliola* Date: 4/25/07

Printed or Typed Name: Robert A. Magliola

My license renewal date is December 31, 2007

Pages or sheets covered by this seal: 1, 2, 2A, 3, 4, 6-44, 51-57

4/25/2007 9:23:50 AM
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 POLK COUNTY - DESIGN NO. 2306

LETTING DATE
IM-235-2 (313) 8--13-77
BRIDGE REPLACEMENT - STEEL GIRDER 10-25-05

POLK COUNTY

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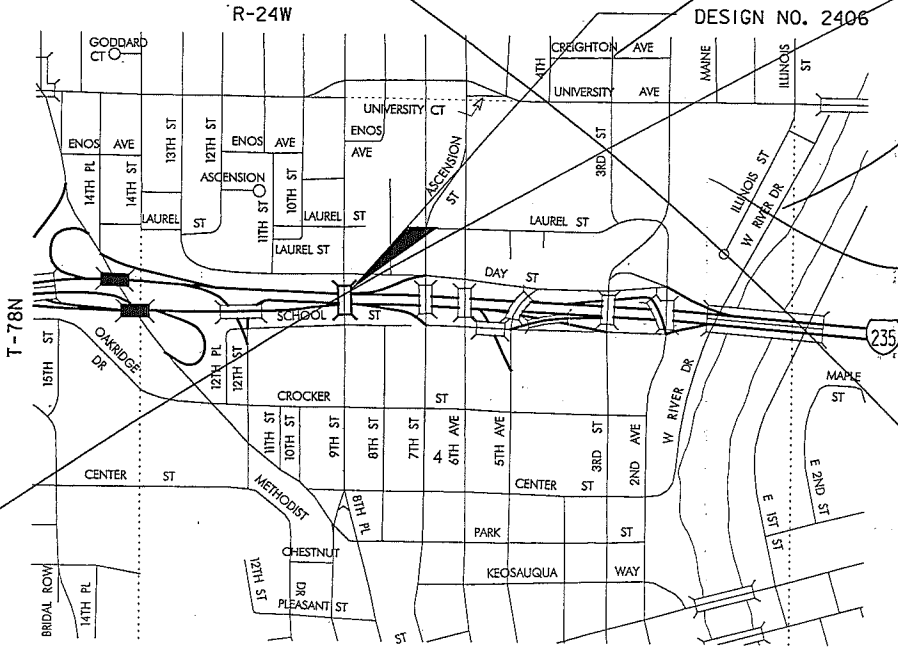


Iowa Department of Transportation
Highway Division

PLANS OF PROPOSED IMPROVEMENTS ON THE
INTERSTATE ROAD SYSTEM
POLK COUNTY
BRIDGE REPLACEMENT - STEEL GIRDER
ON 9th ST. OVER
I-235

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

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PART OF CITY OF DES MOINES

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IM-235-2 (313) 8--13-77	
R.O.W. PROJECT NUMBER	
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99-77-235-332-05	
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REVISIONS	
1/22/07,	SEE SHEET 1A

DESIGN DATA URBAN (I-235)			
1996 AADT	48,500	V.P.D.	
2025 AADT	74,100	V.P.D.	
TRUCKS	8%		

STRUCTURAL DESIGN

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

Robert A. Magliola 1/23/07
Signature Date
Robert A. Magliola

Printed or Typed Name
My license renewal date is December 31, 2007
Pages or sheets covered by this seal: 1, 2, 2A, 3, 4, 6-44, 51-57

THESE PLANS WERE PREPARED FOR THE IOWA DEPARTMENT OF TRANSPORTATION BY: PARSONS
ALL WORKING DRAWINGS INCLUDING SHOP DRAWINGS AND FALSEWORK DRAWINGS WILL BE REVIEWED BY: PARSONS
10 SOUTH RIVERSIDE, SUITE 400 CHICAGO, IL 60606

PROJECT DIRECTORY NAME: 77235180C96 - SECTION 5



Iowa Department of Transportation

Highway Division

PLANS OF PROPOSED IMPROVEMENTS ON THE

INTERSTATE ROAD SYSTEM

POLK COUNTY

BRIDGE REPLACEMENT - STEEL GIRDER ON 9th ST. OVER I-235

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

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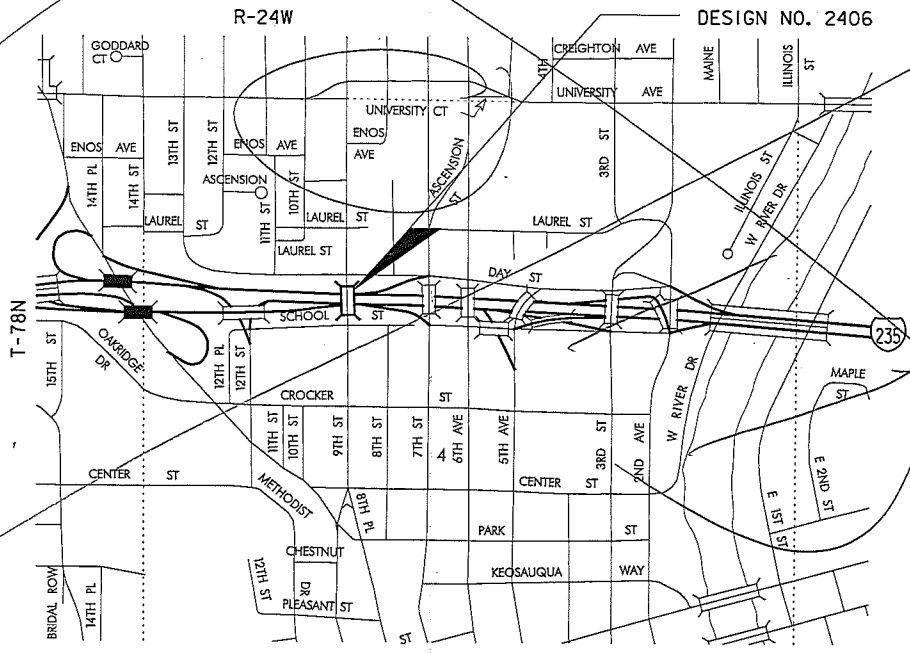
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Robert A. Magliola 6/16/05
 Signature Date
 Robert A. Magliola
 Printed or Typed Name
 My license renewal date is December 31, 2005

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 10 SOUTH RIVERSIDE, SUITE 400
 CHICAGO, IL 60606

PROJECT DIRECTORY NAME: 71235180C96 - SECTION 5

PART OF CITY OF DES MOINES

250

LETTING DATE 10/25/2005
IM-235-2 (313) 8--13-77
BRIDGE REPLACEMENT - STEEL GIRDER

POLK COUNTY
POLK COUNTY - DESIGN NO. 2406
POLK 2466, 2606 W

DESIGN TEAM:	METRIC	IOWA DOT * OFFICE OF BRIDGES AND STRUCTURES	FILE NO. 29552	POLK COUNTY	PROJECT NUMBER IM-235-2 (313) 8--13-77	SHEET NUMBER
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77-235Z-269

GENERAL NOTES :

THIS DESIGN INVOLVES THE CONSTRUCTION OF A 94.0 m x VARIABLE WIDTH CONTINUOUS WELDED PLATE GIRDER BRIDGE ON 9TH STREET OVER I-235.

THE CONSTRUCTION SEQUENCE FOR THIS BRIDGE SHALL BE IN ACCORDANCE WITH THE STAGING NOTE IN THE ROADWAY PLANS, IM-NHS-235-2(269)7--03-77. DELAY IN THE CONSTRUCTION OF CERTAIN BRIDGE COMPONENTS WILL BE REQUIRED TO FACILITATE THE CONSTRUCTION OF THE ADJACENT ROADWAY. REFER TO IM-NHS-235-2(269)7--03-77 FOR THE REQUIRED SEQUENCE.

FAINT LINES ON PLANS INDICATE THE EXISTING STRUCTURE.

THE CITY AND UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE BRIDGE CONTRACTOR OF THE CONSTRUCTION STARTING DATE.

THIS BRIDGE IS DESIGNED FOR MS18 LOADING, PLUS 960 Po FOR FUTURE WEARING SURFACE.

THE BRIDGE CONTRACTOR IS ENCOURAGED TO TAKE FULL ADVANTAGE OF SPECIFICATION 1105.15 -- VALUE ENGINEERING INCENTIVE PROPOSAL PAMPHLET AND CONCEPTUAL PROPOSAL FORM WILL BE AVAILABLE AT THE PRECONSTRUCTION CONFERENCE.

ALL DIMENSIONS IN MILLIMETERS (mm) UNLESS OTHERWISE NOTED OR SHOWN.

ALL ELEVATIONS ON THESE PLANS SHOWN IN METERS (m).

ALL STATIONS SHOWN IN METERS (m).

IT SHALL BE THE BRIDGE CONTRACTOR'S RESPONSIBILITY TO PROVIDE SITES FOR EXCESS EXCAVATED MATERIAL. NO PAYMENT FOR OVERHAUL WILL BE ALLOWED FOR MATERIAL HAULED TO THESE SITES.

SLOUGHING OF EARTH FROM UNDER AN ACTIVE TRAFFIC LANE WILL NOT BE PERMITTED. IF TEMPORARY SHORING (SHEET PILE OR OTHER) IS NECESSARY TO PREVENT THE EARTH UNDER A TRAFFIC LANE, OR AT ANOTHER LOCATION, FROM SLOUGHING IN DURING CONSTRUCTION, THE CONTRACTOR WILL SUBMIT A SHORING PLAN TO THE ENGINEER FOR APPROVAL. COST OF SHORING, IF REQUIRED, WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION AND NO DIRECT PAYMENT WILL BE MADE. ALL MATERIAL USED FOR SHORING SHALL REMAIN THE PROPERTY OF THE CONTRACTOR. SHORING IS TO BE REMOVED ONLY AFTER BACKFILLING HAS BEEN COMPLETED.

SUBSTRUCTURE CONCRETE SHALL BE PROTECTED FROM STAINING BY A WRAPPING OF POLYETHYLENE OR SIMILAR MATERIALS WHICH SHALL BE LEFT IN PLACE AND KEPT IN A SERVICEABLE CONDITION UNTIL AFTER THE DECK HAS BEEN PLACED. IF SUBSTRUCTURE CONCRETE IS STAINED, THE STAINS SHALL BE REMOVED BY METHODS APPROVED BY THE ENGINEER. ALL COSTS ASSOCIATED WITH THE PROTECTION AND ANY REQUIRED CLEANING OF THE SUBSTRUCTURE CONCRETE SHALL BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL STEEL".

THIS STRUCTURE SHALL BE BUILT WITH WEATHERING STEEL. ALL STRUCTURAL STEEL, EXCEPT AS NOTED, SHALL CONFORM TO ASTM A709M GRADE 345W. PAINTING REQUIREMENTS FOR THIS STRUCTURE SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATION 2408.30.

CONCRETE BARRIER RAILS PLACED USING THE SLIP FORM METHOD WILL REQUIRE THE USE OF A CLASS BR CONCRETE IN ACCORDANCE WITH ARTICLE 2513.03B OF THE STANDARD SPECIFICATION. ~~CLASS B CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD.~~

CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. CLASS D CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS (CAST-IN-PLACE OR SLIPFORMED METHOD).

RUSTICATION GROOVE DETAILS ARE SHOWN IN THE PLANS. THE CONTRACTOR SHALL SUBMIT RUSTICATION GROOVE LAYOUTS FOR ALL REQUIRED SURFACES TO THE ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION. ALL COST ASSOCIATED WITH CONCRETE RUSTICATIONS SHALL BE CONSIDERED INCIDENTAL TO THE BID ITEM "HIGH PERFORMANCE STRUCTURAL CONCRETE".

ALL COARSE AGGREGATE FOR STRUCTURAL CONCRETE SHALL BE CRUSHED LIMESTONE.

ORIGINAL DESIGN DRAWINGS INDICATE THAT BATTERED PILES AT EXISTING PIER FOUNDATIONS ARE BATTERED 0.167m/m AND ARE 9.14 m LONG. GIVEN UNKNOWN CONSTRUCTION TOLERANCES AND THE PROXIMITY OF THE FOOTING TO THE DRILLED SHAFTS, THE CONTRACTOR SHOULD BE PREPARED TO ENCOUNTER WOOD PILES DURING DRILLING AND INCLUDE THE COST IN "CONCRETE DRILLED SHAFT, 910 mm DIA."

CONSTRUCTION ACTIVITIES NEAR THE ESTES PROPERTY (944 - 9TH ST.), THE BERRY PROPERTY (1039 - 9TH ST.), AND FORMER FIRE STATION #4 (1041 - 8TH ST.) SHALL NOT CREATE VIBRATIONS IN EXCESS OF A PEAK PARTICLE VELOCITY OF 0.13 INCH/SEC. THE IOWA DOT WILL INSTALL MONITORING DEVICES (SEISMOGRAPHS) AND CONTINUOUSLY MONITOR VIBRATIONS (PEAK PARTICLE VELOCITY) DURING THE CONSTRUCTION PERIOD. THE CONTRACTOR SHALL IMMEDIATELY SHUT DOWN OPERATIONS IF THE PEAK PARTICLE VELOCITY THRESHOLD IS REACHED OR EXCEEDED (VIBRATION EVENT). THE SHUT DOWN WILL REMAIN IN EFFECT UNTIL THE CAUSE OF THE VIBRATION EVENT IS IDENTIFIED AND ALTERNATE EQUIPMENT OR METHODS APPROVED BY THE ENGINEER ARE IN PLACE. THERE WILL BE NO COMPENSATION FOR DOWNTIME AS THE RESULT OF EXCEEDING THE NOTED VIBRATION LIMIT. THERE WILL BE NO COMPENSATION FOR ADJUSTMENT OF PROCESS OR EQUIPMENT TO REDUCE VIBRATION LEVELS TO MEET THE NOTED VIBRATION LIMIT. ALL CONSTRUCTION ACTIVITY AFTER RESUMPTION REMAINS GOVERNED BY THE PREVIOUSLY STATED MAXIMUM PPV VALUE (0.13 INCH/SEC.).

THE CONTRACTOR SHALL PROVIDE A PRE-CONSTRUCTION PLAN 30 DAYS PRIOR TO INITIAL CONSTRUCTION IN THE 9TH ST. AREA. THE PLAN SHALL DESCRIBE THE FOLLOWING:

1. CONSTRUCTION METHODS AND EQUIPMENT THAT WILL BE USED TO MINIMIZE VIBRATION.
2. ALTERNATIVE EQUIPMENT AND METHODS TO BE USED IN CASE OF A VIBRATION EVENT.
3. SITE COMMUNICATIONS METHODS AND EQUIPMENT.
4. DAILY ACTIVITY LOGGING TO ENSURE TIMELY SHUT DOWN AND IDENTIFICATION OF CAUSE.

THE IOWA DOT AND ITS SEISMIC CONSULTANT WILL DETERMINE THE LOCATION OF THE MONITORING DEVICES (SEISMOGRAPHS).

THE CONTRACTOR SHALL PLACE A CHAIN LINK FENCE BETWEEN THE CONSTRUCTION AREA AND THE ESTES PROPERTY (944 - 9TH ST.).

REVISED 4/25/07, SEPARATION BARRIER

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
NOTES & QUANTITIES II
 STATION : 20029+85.670 (@ 9th ST.) OCTOBER 2005
 STATION : 529+85.670 (@ I-235)
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 1A OF 49 FILE NO. 29552 DESIGN NO. 2406

DESIGNED BY MBO CHECKED BY DL
 DETAILED BY FTE CAD FILE RT2406.s01g

POLK COUNTY

PROJECT NUMBER

IM-235-2(313)B--13-77

SHEET NUMBER 2A

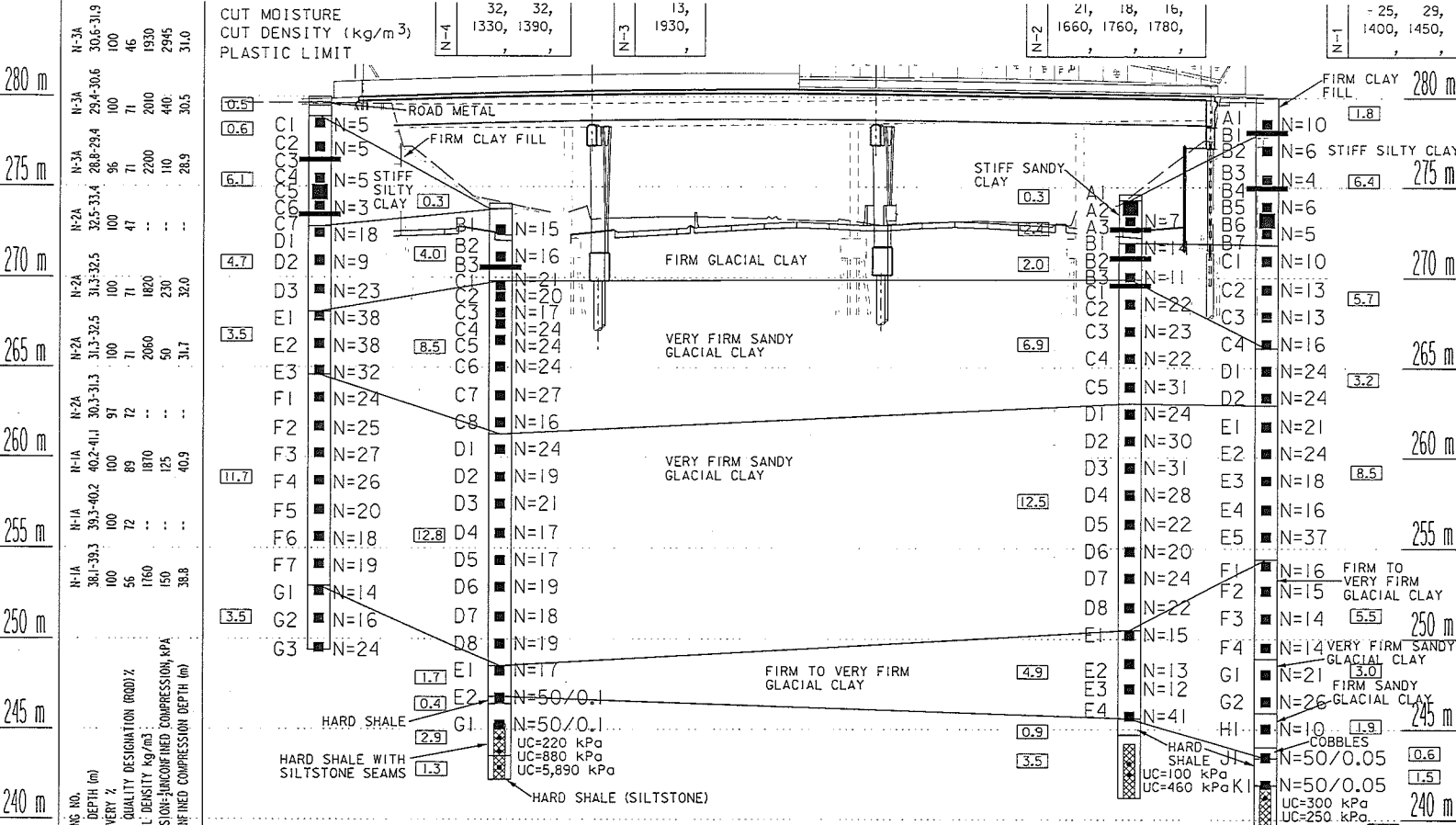
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4/25/2007

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THIS SHEET IS SHOWN TO SHOW SOIL INFORMATION. DETAILS AND NOTES SHOWN ELSEWHERE IN THESE PLANS SHALL BE USED FOR STRUCTURE CONSTRUCTION.



SHELBY TUBE CORE DATA

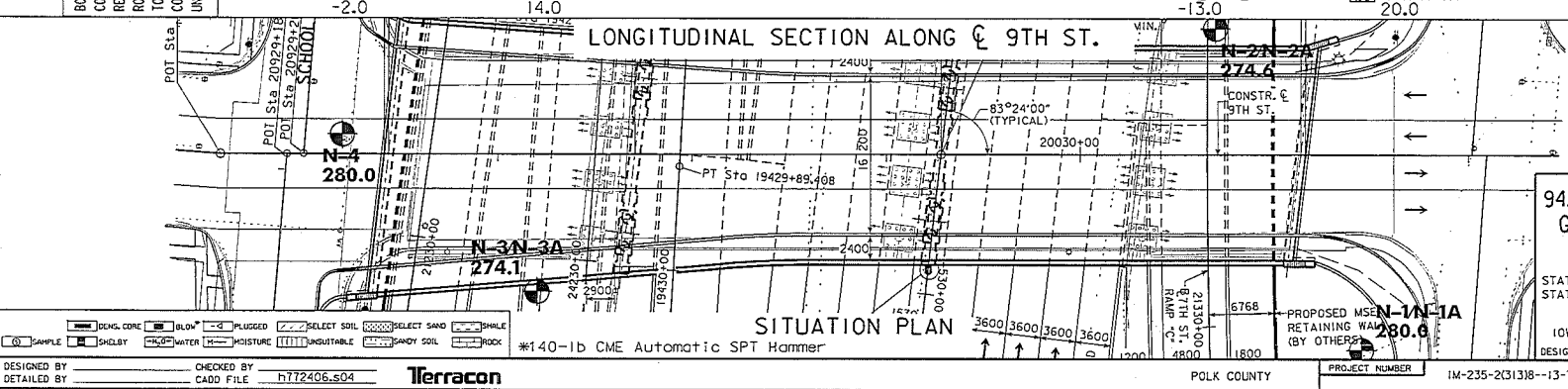
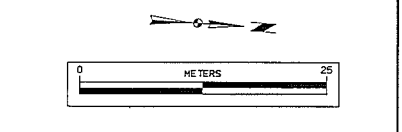
BORING NO.	N-1/N-1A	N-2/N-2A	N-4
DEPTH IN METERS	6.4	0.3	4.9
CLASSIFICATION [AASHTO] A-6(15)	---	---	---
COEFF. CONSOL. m ² /DAY	N/A	---	---
TRIAxIAL COMPRESSION	CU	UC	UC
COHESION - kPa	28	60	20
FRICTION COEFF.	0.291	---	---
MOISTURE CONTENT %	28	22	31
DENSITY - kg/m ³	1540	1600	1390
UU-UNCONSOLIDATED & UNDRAINED			
CU-CONSOLIDATED & UNDRAINED			
UC-UNCONFINED COMPRESSION (C=1/2 Qu)			

BLOW COUNT
LAYER - N. BLOWS

B2 = 5

█ - INDICATES LAYER THICKNESS (m)

LOCATION
9TH ST. OVER I-235
T-78 N R-24 W
SECTION 4
DES MOINES TOWNSHIP
POLK COUNTY
BRIDGE MAINT. NO. 7708.10235
FHWA NO. 042571



GEOTECHNICAL DESIGN

I hereby certify that this plan was prepared under my supervision and that engineering decisions with regard to the design were made by me or by other duly licensed Professional Engineers under the laws of the State of Iowa.

Signature: *Kyle C. Berg* Date: 6/13/05
 Printed or Typed Name: KYLE C. BERG
 My license renewal date is December 31, 2005

Pages or sheets covered by this seal: SPS_01

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.30m, 31.60m, 37.10m SPANS
SOILS PROFILE SHEET
 STATION : 20029+85.670 (9th St.)
 STATION : 529+85.570 (9th St.)
 APRIL 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 4 of 49 FILE NO. 29552 DESIGN NO. 2406

PROJECT NUMBER: IM-235-231318--13-77
 SHEET NUMBER: 5

PIER REINFORCING BAR LIST - PIER NO. 1

MARK	SIZE	LOCATION	SHAPE	NO.	LENGTH	MASS
o1	35	CAP, TOP, LONGITUDINAL		32	14 330	3 600
o2	25	CAP, SIDE, LONGITUDINAL		4	12 970	204
o3	25	CAP, SIDE, LONGITUDINAL		4	12 890	202
o4	25	CAP, SIDE, LONGITUDINAL		4	12 790	201
o5	25	CAP, SIDE, LONGITUDINAL		4	12 690	199
o6	35	CAP, BOTTOM, LONGITUDINAL		16	16 820	2 113
o7	35	CAP, BOTTOM, LONGITUDINAL		16	11 010	1 383
c1	15	CAP, HOOPS, CANTILEVER	□	4	2 960	19
c2	15	CAP, STIRRUPS	□	200	2 320	728
c3	20	CAP, STIRRUPS	□	228	2 350	1 262
e8	15	SHAFT SPIRAL		4	217 160	1 364
		L22x22x3.2 SPIRAL SPACER (1.0 kg/m)		16	25 680	427
f1	25	FOOTING, TOP AND SIDES		28	4 300	473
f2	30	FOOTING, BOTTOM LONGITUDINAL		24	5 300	699
f3	15	FOOTING, STIRRUPS	□	40	3 225	203
f4	30	SHAFT, LONGITUDINAL		28	14 500	2 231
f5	30	SHAFT, LONGITUDINAL		28	11 450	1 762
f6	30	SHAFT, LONGITUDINAL		28	15 240	2 345
f7	30	SHAFT, LONGITUDINAL		28	18 280	2 813
m1	15	CAP, STEP, LONGITUDINAL		16	960	24
n1	15	CAP, STEP, TRANSVERSE		20	1 580	50
t1	15	CAP, TRANSVERSE		54	1 510	128
				TOTAL NON-COATED (kg)	22 430	
c1	30	STEM, VERTICAL		76	7 595	3 172
o2	30	STEM, VERTICAL		8	6 365	280
o3	30	STEM, VERTICAL		60	7 515	2 478
d4	30	FOOTING TO STEM DOWELS		12	4 680	309
d5	30	FOOTING TO STEM DOWELS		116	3 800	2 422
e1	15	STEM HOOPS	□	28	5 540	244
e2	15	STEM HOOPS	□	16	6 880	173
e3	15	COLUMN HOOPS	□	36	VARIES	320
e4	15	COLUMN HOOPS	□	24	VARIES	211
e5	15	COLUMN HAIRPINS	□	40	VARIES	220
e6	15	STEM HOOPS	□	2	7 400	23
e7	15	STEM HOOPS	□	2	7 900	25
g1	15	STEM TIE		28	1 520	67
g2	15	COLUMN TIE		68	1 660	177
g3	15	COLUMN AND STEM TIE		36	VARIES	90
g4	15	COLUMN TIE		24	VARIES	58
				TOTAL EPOXY COATED (kg)	10 269	

PIER REINFORCING BAR LIST - PIER NO. 2

MARK	SIZE	LOCATION	SHAPE	NO.	LENGTH	MASS
d8	35	CAP, TOP, LONGITUDINAL		32	13 180	3 311
o9	25	CAP, SIDE, LONGITUDINAL		4	11 830	186
o10	25	CAP, SIDE, LONGITUDINAL		4	11 750	184
o11	25	CAP, SIDE, LONGITUDINAL		4	11 650	183
o12	25	CAP, SIDE, LONGITUDINAL		4	11 550	181
o13	35	CAP, BOTTOM, LONGITUDINAL		16	14 520	1 824
o14	35	CAP, BOTTOM, LONGITUDINAL		16	11 010	1 383
c1	15	CAP, HOOPS, CANTILEVER	□	4	2 960	19
c2	15	CAP, STIRRUPS	□	252	2 320	918
c3	20	CAP, STIRRUPS	□	176	2 350	974
e9	15	SHAFT SPIRAL		4	244 090	1 533
		L22x22x3.2 SPIRAL SPACER (1.0 kg/m)		16	30 100	482
f1	25	FOOTING, TOP AND SIDES		28	4 300	473
f2	30	FOOTING, BOTTOM LONGITUDINAL		24	5 300	699
f3	15	FOOTING, STIRRUPS	□	40	3 225	203
f8	30	SHAFT, LONGITUDINAL		28	17 930	2 759
f9	30	SHAFT, LONGITUDINAL		28	14 890	2 291
f10	30	SHAFT, LONGITUDINAL		28	15 240	2 345
f11	30	SHAFT, LONGITUDINAL		28	18 280	2 813
m1	15	CAP, STEP, LONGITUDINAL		12	960	18
n1	15	CAP, STEP, TRANSVERSE		15	1 580	37
t1	15	CAP, TRANSVERSE		48	1 510	114
				TOTAL NON-COATED (kg)	22 930	
d6	30	STEM, VERTICAL		76	7 285	3 042
d7	30	STEM, VERTICAL		8	6 055	266
d8	30	STEM, VERTICAL		60	7 205	2 375
d9	30	FOOTING TO STEM DOWELS		12	4 370	288
d10	30	FOOTING TO STEM DOWELS		116	3 500	2 231
e1	15	STEM HOOPS	□	24	5 540	209
e2	15	STEM HOOPS	□	14	6 880	151
e3	15	COLUMN HOOPS	□	36	VARIES	320
e4	15	COLUMN HOOPS	□	24	VARIES	211
e5	15	COLUMN HAIRPINS	□	40	VARIES	220
e6	15	STEM HOOPS	□	2	7 400	23
e7	15	STEM HOOPS	□	2	7 900	25
g1	15	STEM TIE		24	1 520	57
g2	15	COLUMN TIE		68	1 660	177
g3	15	COLUMN AND STEM TIE		36	VARIES	90
g4	15	COLUMN TIE		24	VARIES	58
				TOTAL EPOXY COATED (kg)	9 743	

H.P. STRUCTURAL CONCRETE PLACEMENT QUANTITIES - TWO PIERS

LOCATION	PIER NO. 1	PIER NO. 2	QUANTITY
CAP AND STEPS	36.0	32.6	68.6
STEM/COLUMNS	56.5	53.7	110.2
TOTAL - m ³	92.5	86.3	178.8

CONCRETE PLACEMENT QUANTITIES - TWO PIERS

LOCATION	PIER NO. 1	PIER NO. 2	QUANTITY
FOOTINGS	[22.7]	[22.7]	[45.5]
TOTAL - m ³			[45.5]

ESTIMATED QUANTITIES - TWO PIERS

ITEM	UNIT	PIER NO. 1	PIER NO. 2	TOTAL
HIGH PERFORMANCE STRUCTURAL CONCRETE	m ³	92.5	86.3	178.8
STRUCTURAL CONCRETE (BRIDGE)	m ³	[22.7]	[22.7]	[45.5]
REINFORCING STEEL-EPOXY COATED	kg	10 269	9 743	20 012
REINFORCING STEEL	kg	22 430	22 930	45 360
EXCAVATION CLASS 20	m ³	115.0		115.0
CONCRETE DRILLED SHAFT, 910 DIA.	m	107.20	120.93	228.13

PIER NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 50 mm UNLESS OTHERWISE NOTED OR SHOWN.

ALL EXPOSED CORNERS 90° OR SHARPER TO BE FILLETED WITH A 20 mm DRESSED OR BEVELED STRIP.

ALL REINFORCING IS TO BE SECURELY WIRED IN PLACE BEFORE CONCRETE IS POURED. FORMS FOR DIAGONAL PIER COLUMNS SHALL BE ADEQUATELY BRACED TO PREVENT SPREADING OR DEFLECTION OF THE COLUMNS PRIOR TO THE PLACING AND CURING OF THE CAP.

THE USE OF STEEL FORMS IS REQUIRED FOR THE FORMING OF ALL PIER CONCRETE SURFACES FROM THE TOPS OF FOOTINGS TO THE BOTTOM OF PIER CAP BEAM, INCLUDING STEM AND PIER COLUMNS. USE OF MEDIUM-DENSITY OVERLAP (MDO) OR HIGH-DENSITY OVERLAP (HDO) PLYWOOD FACED FORMS IS ALLOWED FOR FORMING OF THE PIER CAP BEAM AND COLUMN PROJECTIONS IN THE REGION OF THE PIER CAP. FLAIN PLYWOOD-FACED FORMS WILL NOT BE ALLOWED FOR ANY PORTION OF THE PIER COLUMN OR CAP SURFACES.

THE SPIRAL REINFORCING IN THE DRILLED SHAFT MAY BE SPLICED BY LAPPING 600 mm. THE LENGTH OF SPIRAL SHOWN DOES NOT INCLUDE THE LAPPED LENGTH OF SPLICES. THE COST OF THE LAPS AT SPLICES IS TO BE INCLUDED IN THE PRICE BID FOR REINFORCING.

THE FORMS FOR THE SHAFT, COLUMN AND CAP CONCRETE SHALL BE LEFT IN PLACE FOR A MINIMUM OF 5 DAYS. FORM REMOVAL SHALL BE IN ACCORDANCE WITH ARTICLE 2403.18 OF THE STANDARD SPECIFICATIONS.

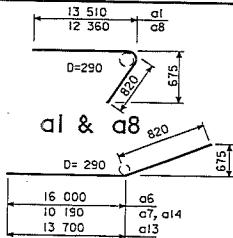
THE DESIGN CAPACITY FOR ALL PIER DRILLED SHAFTS = 4200 KN. THIS CAPACITY IS BASED ON WORKING STRESS DESIGN.

THE MASS OF ANCHOR BOLTS IS INCLUDED IN THE STRUCTURAL STEEL QUANTITY.

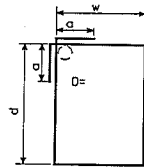
REVISED 1/22/07, FOOTING CONCRETE QUANTITY

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
PIER DETAILS
 STATION : 20029+85.670 (C 9th ST.)
 STATION : 529+85.670 (C 1-235)
 OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 10 OF 49 FILE NO. 23552 DESIGN NO. 2406

BENT BAR DETAILS

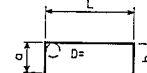


d6, a7, a13 & a14



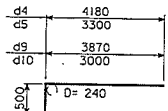
c1, e1, e2, e3, e4, e6 & e7

	a	d	w	D=
c1	150	650	880	60
e1	150	1380	1240	60
e2	150	1380	1910	60
e3	150	1380	1360 TO 1250	60
e4	150	1380	1310 TO 1220	60
e6	150	1380	2170	60
e7	150	1380	2420	60

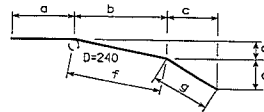


c2, c3, f2, f3, e5, n1 & t1

	L	a	b	D=
c2	880	820	820	60
c3	890	830	830	120
f2	4300	500	500	240
f3	1525	850	850	60
e5	1200 TO 1140	1170	1170	60
n1	740	200	470	60
t1	1200	155	155	60

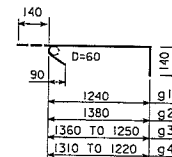


d4, d5, d9 & d10



	a	b	c	d	e	f	g
d1	2210	2220	2400	1000	1715	2435	2950
d2	2210	2220	1400	1000	1000	2435	1720
d3	1910	2220	2700	880	1740	2390	3215
d6	1900	2220	2400	1000	1715	2435	2950
d7	1900	2220	1400	1000	1000	2435	1720
d8	1600	2220	2700	880	1740	2390	3215

d1, d2, d3, d6, d7 & d8



g1 to g4

NOTE : ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.

DESIGNED BY MBO CHECKED BY DL
 DETAILED BY FJE CADD FILE I772406.dwg

PIER REINFORCING BAR LIST - PIER NO. 1

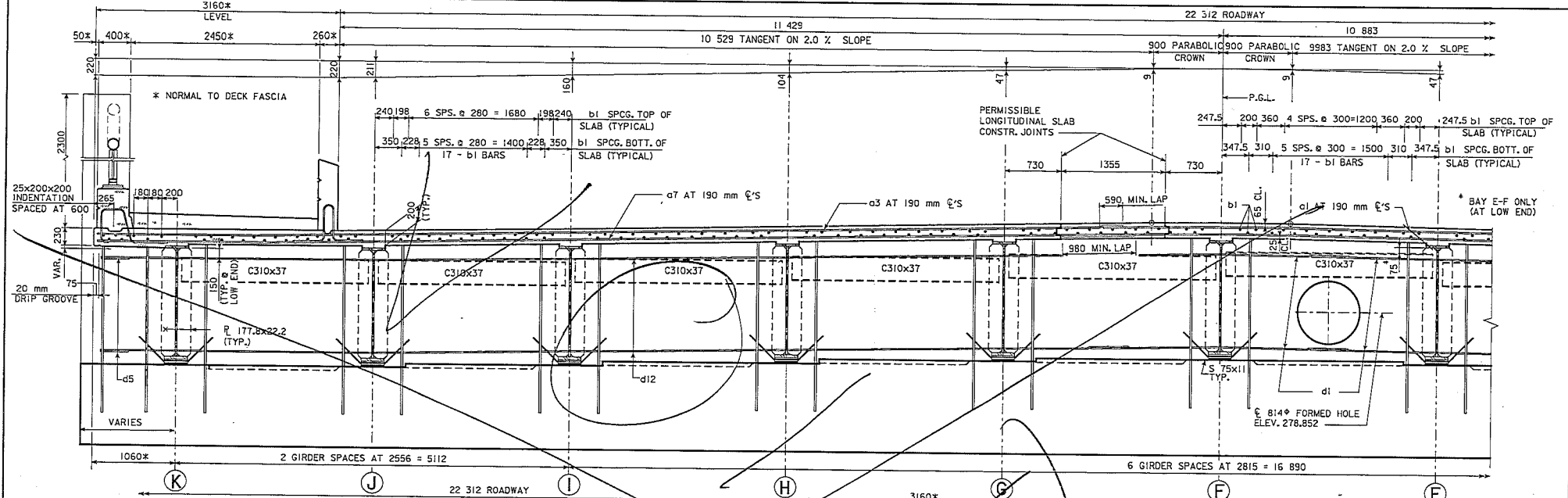
POLK COUNTY

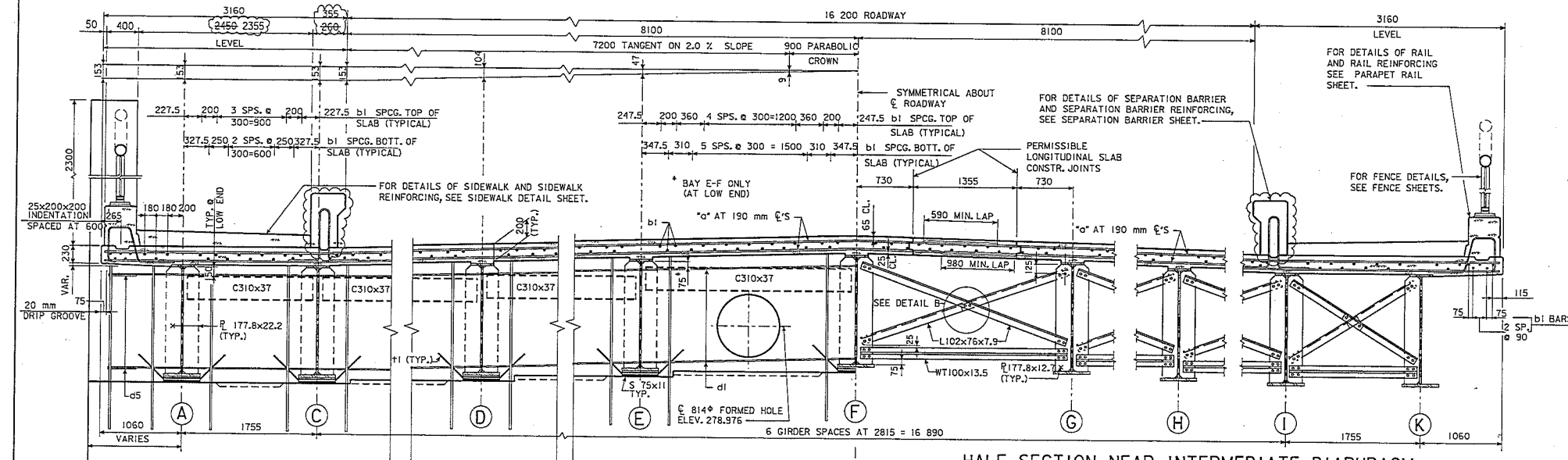
PROJECT NUMBER

IM-235-2(3)318--13-77

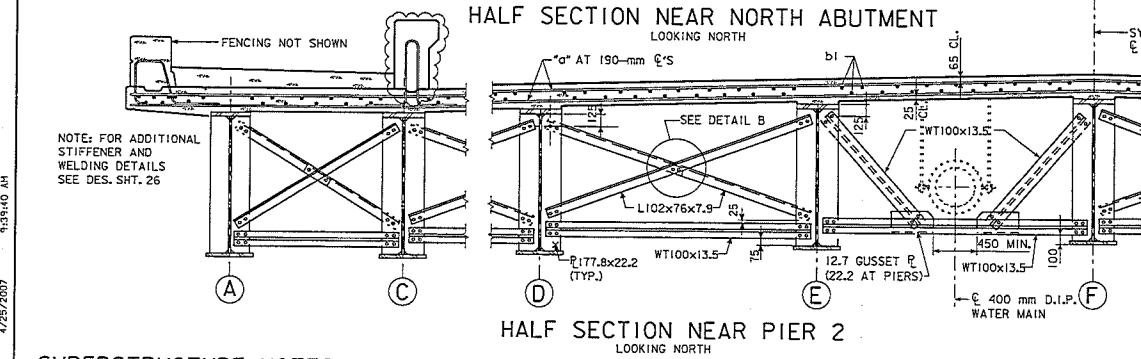
SHEET NUMBER

11



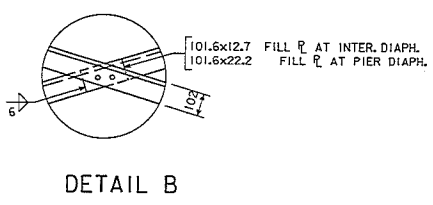


HALF SECTION NEAR INTERMEDIATE DIAPHRAGM
LOOKING NORTH

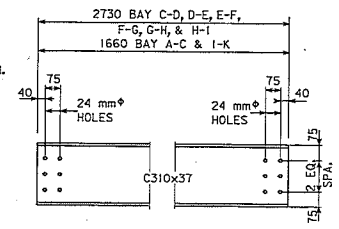


HALF SECTION NEAR NORTH ABUTMENT
LOOKING NORTH

HALF SECTION NEAR PIER 2
LOOKING NORTH



DETAIL B

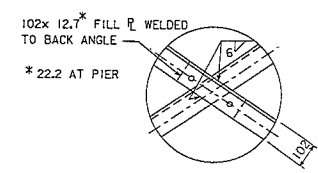


NORTH ABUT. DIAPHRAGM DETAIL

SUPERSTRUCTURE NOTES:

THE FLOOR SLAB AS SHOWN INCLUDES 13 mm INTEGRAL WEARING SURFACE.
FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE GIRDERS.
CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 50 mm UNLESS OTHERWISE NOTED OR SHOWN.
TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 65 mm CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 25 mm CLEAR ABOVE BOTTOM OF SLAB.
TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 900 mm CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 1200 mm APART.
ALL REINFORCING BARS ARE TO BE EPOXY COATED.
ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE 24 mm Ø AND ALL BOLTS ARE TO BE 22.2 mm Ø.

BOTTOM FLANGES ARE TO BE PERPENDICULAR TO WEBS AT THE REACTION POINTS.
FILL PLATE THICKNESSES SHOWN ON PLANS ARE BASED ON NOMINAL GIRDER DIMENSIONS. THESE THICKNESSES ARE TO BE VERIFIED OR ADJUSTED DURING FABRICATION TO SECURE A CLOSE FIT. EACH FILL PLATE SHALL FIT TO THE NEAREST 2 mm IN THICKNESS AND SINGLE PLATES ARE REQUIRED AT EACH FILL LOCATION. GIRDERS ARE TO BE TRULY SQUARE AT SPLICE POINTS WITH FLANGES PERPENDICULAR TO WEBS.
THE DESIGN DRAWINGS INDICATE AWS PREQUALIFIED WELDED JOINTS. ALTERNATE JOINT DETAILS MAY BE SUBMITTED FOR APPROVAL.
MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, WILL BE REQUIRED.

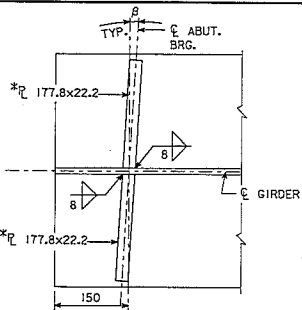


ALTERNATE DETAIL B

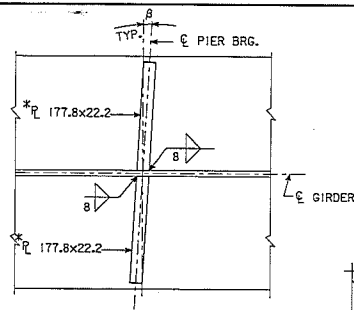
NOTE: USE ALTERNATE AT LOCATIONS WHERE GIRDER SPACING IS LESS THAN 2815.

REVISED 4/25/07, SEPARATION BARRIER

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
SUPERSTRUCTURE DETAILS
 STATION : 20029+85.670 (C 9th ST.)
 STATION : 529+85.670 (C 1-235)
 OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 18 OF 49 FILE NO. 29552 DESIGN NO. 2406



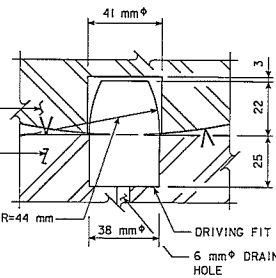
ABUTMENT BEARING STIFFENERS



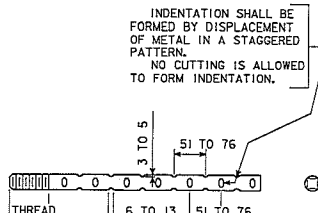
PIER BEARING STIFFENERS

* BEVEL GIRDER J AND GIRDER K BEARING STIFFENER PLATES TO FIT AGAINST WEB AT S. ABUTMENT AND PIER 1.

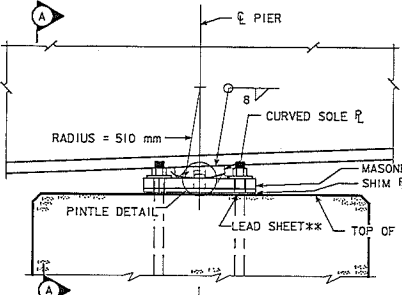
TABLE OF β VALUES				
BEARING LOCATION	S. ABUT.	PIER 1	PIER 2	N. ABUT.
GIRDER A	2°38'13"	2°38'13"	6°36'00"	6°36'00"
GIRDER B	4°37'28"	4°37'28"	--	--
GIRDER C THRU I	6°36'00"	6°36'00"	6°36'00"	6°36'00"
GIRDER J	8°59'15"	8°59'15"	--	--
GIRDER K	11°20'33"	11°20'33"	6°36'00"	6°36'00"



PINTE DETAIL

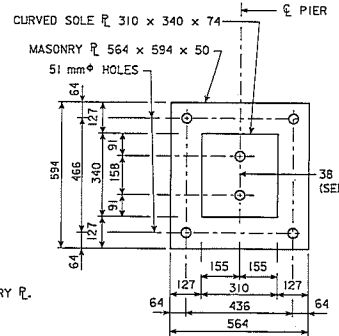


ANCHOR BOLT SWEDGE DETAIL

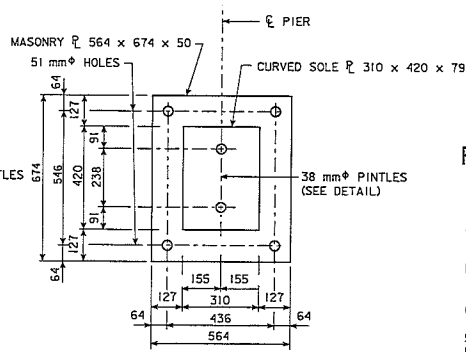


PART ELEVATION AT PIER (LOOKING WEST)

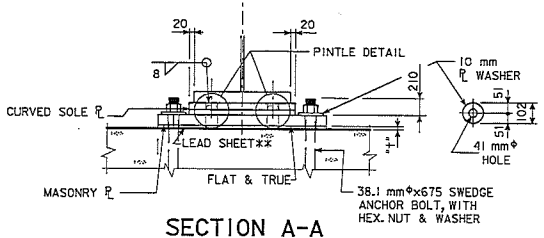
** LEAD SHEET AND SHIM PLATE SHALL BE SAME PLAN DIMENSION AS MASONRY PLATE. LEAD SHEET SHALL BE 3mm THICK. SEE TABLE FOR SHIM PLATE THICKNESS.



PLAN VIEW OF MASONRY AND SOLE PLATES PIER 1



PLAN VIEW OF MASONRY AND SOLE PLATES PIER 2



SECTION A-A

TABLE OF SHIM PLATE THICKNESS (mm)

GIRDER	A	B	C	D	E	F	G	H	I	J	K
PIER 1	27	26	39	0	0	0	0	81	22	2	0
PIER 2	0	-	5	57	0	0	0	64	17	-	13

ELEVATION OF CURVED SOLE PLATE PIER 1

ELEVATION OF CURVED SOLE PLATE PIER 2

FIXED PIER BEARINGS

WEATHERING STEEL NOTES :

ALL STRUCTURAL STEEL, EXCEPT AS NOTED, SHALL CONFORM TO ASTM A709M GRADE 345W. THE MINIMUM YIELD POINT FOR GRADE 345W STRUCTURAL STEEL IS 345 MPa FOR PLATES 100 mm AND UNDER IN THICKNESS, AND ALL STRUCTURAL SHAPES. THE GRADE 345W STEEL IS A WEATHERING STEEL AND IS TO REMAIN UNPAINTED, EXCEPT AS NOTED.

FLANGE DEFLECTORS AS DETAILED ON DESIGN SHTS. 19 & 26 ARE TO BE GRADE 250 OR 345W.

ALL PIECES COMPRISING THE ABUTMENT AND PIER BEARINGS SHALL COMPLY WITH THE REQUIREMENTS AS STATED IN THE NOTES ON THIS SHEET.

SHEAR STUDS ARE TO BE OF AN APPROVED TYPE LISTED IN MATERIALS I.M. 453.10, APPENDIX A.

THE FINISH ON BEARINGS, FLANGE DEFLECTORS AND WEATHERING STEEL SHALL BE IN ACCORDANCE WITH THE PLAN NOTES AND STANDARD SPECIFICATION 2408. ALL WEATHERING STEEL EMBEDDED INTO AN INTEGRAL ABUTMENT SHALL BE PAINTED TO A DISTANCE OF 300 mm FROM THE CONCRETE FACE AND SEALED BY CAULKING AT THE ABUTMENT CONCRETE AND STEEL INTERFACE.

THE GRADE 345W STEEL FOR THE WEBS OF THE EXTERIOR GIRDERS OF THE BRIDGE SHALL BE OF THE SAME TYPE AND FROM THE SAME SOURCE.

BOLTS FOR USE WITH WEATHERING STEEL SHALL BE A325 TYPE III WITH A563 GRADE DH3 NUTS AND F436 TYPE III WASHERS.

BOLTS USED TO SPLICE GIRDER SECTIONS ARE TO BE INSTALLED SUCH THAT NUTS ARE ON THE INSIDE FACE OF THE GIRDER WEBS FOR THE EXTERIOR GIRDERS, AND ON THE TOP OF BOTH TOP AND BOTTOM FLANGES OF ALL THE GIRDERS.

THE STEEL SHALL BE KEPT FREE OF OIL, GREASE, DIRT, CRAYON OR CHALK MARKS, CONCRETE SPATTER AND ANY OTHER FOREIGN MATTER THAT MAY AFFECT THE NATURAL OXIDATION OF THE STEEL. ANY FOREIGN MATTER REMAINING ON THE STEEL AFTER COMPLETION OF BRIDGE CONSTRUCTION SHALL BE REMOVED BY THE BRIDGE CONTRACTOR AS DIRECTED BY THE ENGINEER. THE RESULTANT SURFACE SHALL BE FREE OF ALL VISIBLE RESIDUES. ALL COSTS ASSOCIATED WITH CLEANING STEEL SURFACES SHALL BE BORNE BY THE BRIDGE CONTRACTOR.

SEAL MATERIAL FOR CAULKING SHALL BE NEUTRAL CURE AND NON SAG SILICONE. THREE PRODUCTS MEETING THESE CRITERIA ARE DOW 888, CSL342 AND CRAFTO ROADS AVER JOINT SEALANT.

TO ENSURE UNIFORM WEATHERING, ALL UNPAINTED AREAS OF OUTSIDE SURFACES OF THE FACIA GIRDERS SHALL RECEIVE, AFTER BLASTING, AT LEAST THREE UNIFORM APPLICATIONS OF WATER MIST WITH A 24 HOUR INTERVAL BETWEEN APPLICATIONS. EACH APPLICATION SHALL BE APPLIED ON DRY SURFACES. THE WATER MIST APPLICATION SHALL BE PERFORMED WITHIN 48 HOURS AFTER THE PAINTED SURFACES HAVE BEEN PROPERLY CURED. ALL WATER MIST APPLICATIONS SHALL BE WITNESSED BY A REPRESENTATIVE OF THE CONTRACTING AUTHORITY.

PIER BEARING NOTES:

SURFACES MARKED "V" SHALL BE FINISHED 6 μ m.

MASONRY PLATES ARE TO BE SET ON A 3 mm LEAD SHEET.

SOLE PLATES, MASONRY PLATES, AND LEAD SHEETS ARE A PART OF THE SUPERSTRUCTURE STRUCTURAL STEEL QUANTITY.

MASONRY PLATES SHALL BE GALVANIZED.

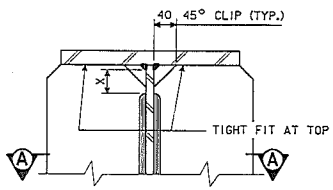
CURVED SOLE PLATES SHALL COMPLY WITH ASTM A 709M GRADE 485W AND PAINTED AS PER STANDARD SPECIFICATIONS.

MASONRY PLATES SHALL COMPLY WITH ASTM A 709M GRADE 485.

ANCHOR BOLTS, NUTS AND WASHERS SHALL MEET THE REQUIREMENTS OF IM 453.08.

THE CONTRACTOR WILL BE ALLOWED TO SUBSTITUTE NEOPRENE SHEETS WITH 50 DUREMETER HARDNESS IN PLACE OF THE LEAD SHEET ON THE BEARING DETAILS. PAYMENT FOR STRUCTURAL STEEL WILL INCLUDE NO DEDUCTION IN STEEL WEIGHT DUE TO ELIMINATION OF THE LEAD SHEETS AND/OR NO ADDITIONAL COSTS ASSOCIATED WITH THE ADDITION OF THE NEOPRENE SHEETS.

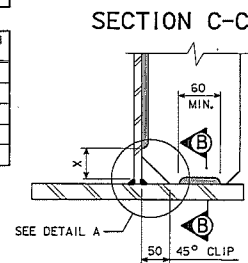
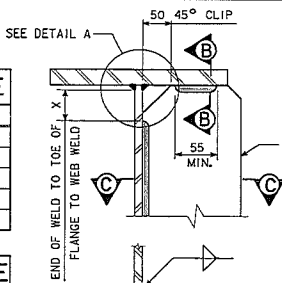
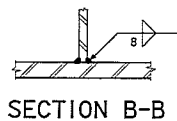
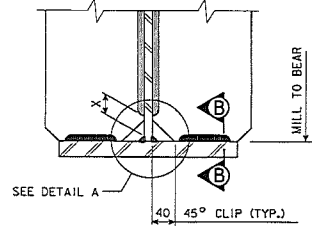
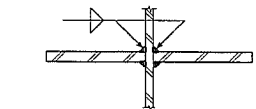
DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
SUPERSTRUCTURE DETAILS
 STATION : 20029+85.670 (E 9th ST.)
 STATION : 529+85.670 (E 1-235) OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 25 OF 49 FILE NO. 29552 DESIGN NO. 2406



BEARING STIFFENER TABLE	
LOCATION	PLATE SIZE
SOUTH ABUTMENT	177.8 x 22.2
PIER 1	177.8 x 22.2
PIER 2	177.8 x 22.2
NORTH ABUTMENT	177.8 x 22.2

FLANGE TO WEB WELD SIZE	
SIZE OF FILLET WELD	FLANGE THICKNESS
8	ALL FLANGES

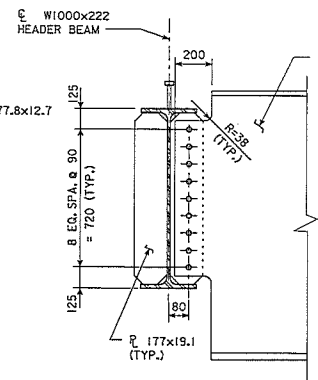
T - WEB THICKNESS	x = ST WITH 60 mm MIN.
12	60
14	70



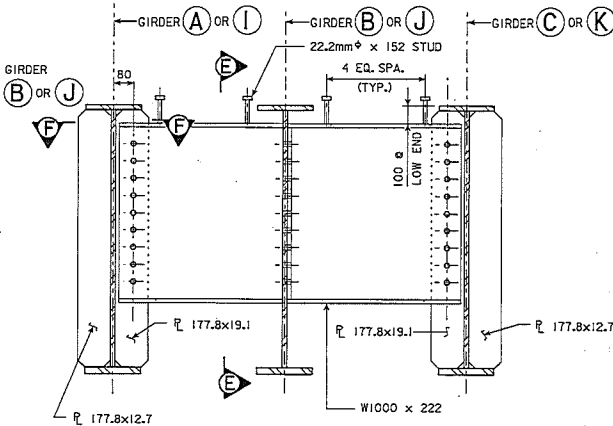
ABUTMENT AND PIER BEARING STIFFENERS (PARALLEL TO ϕ OF BEARING)

INTERMEDIATE DIAPHRAGM STIFFENER

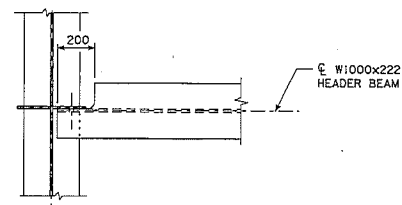
NOTE: BEVEL GIRDER A, B, J AND K INTERMEDIATE DIAPHRAGM STIFFENER PLATES TO FIT AGAINST THE WEB AT LOCATIONS BETWEEN THE SOUTH ABUTMENT AND THE HEADER.



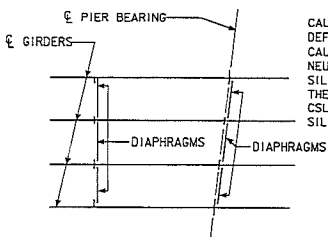
SECTION E-E (WELDING PER INTERMEDIATE DIAPHRAGM STIFFENER DETAIL)



HEADER BEAM ELEVATION (SEE STRUCTURAL STEEL LAYOUT FOR LOCATIONS)



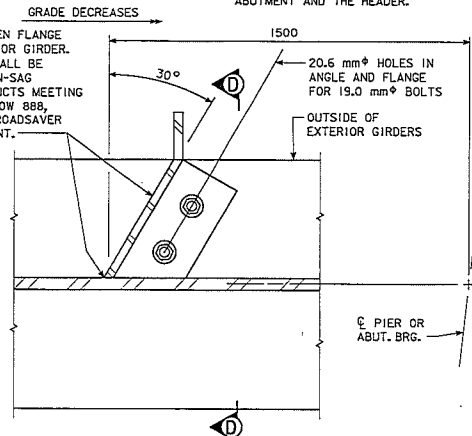
SECTION F-F



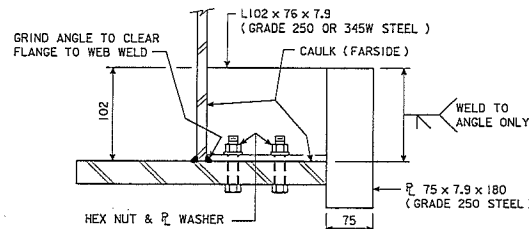
DIAPHRAGM FRAMING

FLANGE DEFLECTORS ARE REQUIRED ON THE OUTSIDE OF THE EXTERIOR GIRDERS AT THE ABUTMENTS AND PIERS. FOR LOCATION OF FLANGE DEFLECTORS SEE DESIGN SHEET 19.

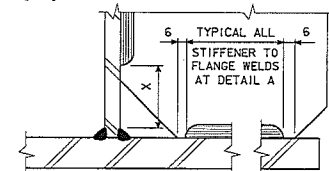
FLANGE DEFLECTOR COMPONENTS WITH GRADE 250 STEEL ARE TO BE PAINTED IN ACCORDANCE WITH STANDARD SPECIFICATIONS. WEATHERING STEEL 345W IS TO REMAIN UNPAINTED.



FLANGE DEFLECTOR DETAILS (8 REQUIRED PER BRIDGE)



SECTION D-D



DETAIL A

NOTE: THIS SHEET IS PRIMARILY FOR THE USE OF FABRICATOR'S WORKMEN AND IOWA DEPARTMENT OF TRANSPORTATION INSPECTORS IN INTERPRETING PLAN DETAILS. IT COVERS THE LOCATIONS OF WELD TERMINI THAT ARE NOT SPECIFIED BY TYPICAL WELD SYMBOLS.

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25,300m, 31,600, 37,100 SPANS
SUPERSTRUCTURE DETAILS
 STATION : 20029+85.670 (ϕ 9th ST.)
 STATION : 529+85.670 (ϕ 1-235)
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 26 OF 49 FILE NO. 29552 DESIGN NO. 2406

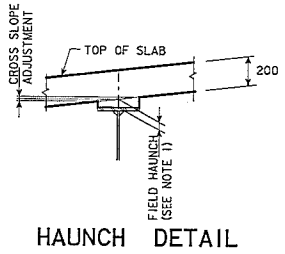
REVISED 10-00 - FLANGE DEFLECTOR DETAIL ADDED. LONGLY STIFFENER, SUSSET PLATE TO WEB AND SECTION G-G MOVED OUTSIDE OF BORDER.

TABLE OF BEAM LINE HAUNCH ELEVATIONS

BEAM LINE	€ S. ABUT. BRG.								€ PIER 1										€ PIER 2					€ N. ABUT. BRG.								
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23		24	25	26	27	28	29	30	31
A	279.593	279.670	279.739	279.800	279.851	279.894	279.929	279.954	279.976	280.004	280.030	280.049	280.061	280.067	280.067	280.061	280.051	280.039	280.025	280.013	280.001	279.991	279.980	279.967	279.949	279.927	279.898	279.865	279.826	279.783	279.739	
B	279.591	279.667	279.737	279.798	279.850	279.893	279.928	279.952	279.975	280.003	280.028	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	
C	279.631	279.707	279.774	279.832	279.880	279.918	279.949	279.969	279.987	280.011	280.032	280.047	280.059	280.065	280.065	280.059	280.049	280.038	280.026	280.017	280.009	280.002	279.994	279.984	279.968	279.946	279.917	279.880	279.838	279.790	279.740	
D	279.681	279.757	279.825	279.884	279.932	279.971	280.001	280.022	280.041	280.065	280.087	280.102	280.112	280.116	280.115	280.109	280.100	280.089	280.078	280.069	280.060	280.054	280.046	280.036	280.026	280.016	280.006	279.996	279.932	279.890	279.842	279.792
E	279.731	279.807	279.876	279.935	279.984	280.023	280.054	280.075	280.094	280.119	280.141	280.157	280.168	280.172	280.171	280.166	280.157	280.146	280.136	280.127	280.119	280.113	280.105	280.094	280.079	280.057	280.027	279.991	279.949	279.901	279.851	
F	279.772	279.848	279.917	279.977	280.026	280.066	280.098	280.119	280.139	280.164	280.187	280.203	280.214	280.219	280.219	280.214	280.205	280.195	280.185	280.177	280.169	280.162	280.155	280.144	280.128	280.106	280.077	280.041	279.998	279.951	279.901	
G	279.718	279.795	279.865	279.925	279.974	280.015	280.047	280.069	280.089	280.114	280.137	280.154	280.166	280.171	280.171	280.167	280.159	280.149	280.140	280.132	280.124	280.118	280.110	280.099	280.084	280.062	280.032	279.996	279.954	279.906	279.856	
H	279.655	279.732	279.803	279.863	279.913	279.954	279.987	280.009	280.030	280.056	280.079	280.097	280.109	280.115	280.115	280.111	280.103	280.094	280.085	280.078	280.071	280.064	280.056	280.046	280.030	280.008	279.979	279.942	279.900	279.852	279.802	
I	279.593	279.670	279.741	279.802	279.852	279.894	279.927	279.949	279.970	279.997	280.021	280.039	280.053	280.062	280.065	280.062	280.055	280.046	280.038	280.031	280.024	280.017	280.009	279.999	279.983	279.961	279.932	279.896	279.853	279.806	279.755	
J	279.536	279.615	279.688	279.752	279.809	279.857	279.897	279.925	279.951	279.984	280.014	280.037	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	
K	279.521	279.602	279.679	279.746	279.803	279.852	279.893	279.922	279.949	279.983	280.014	280.038	280.054	280.063	280.067	280.064	280.058	280.048	280.039	280.030	280.020	280.010	279.998	279.985	279.968	279.945	279.917	279.883	279.844	279.802	279.757	

MISCELLANEOUS DATA TABLE (mm)

	BEAM LINE	€ S. ABUT. BRG.								€ PIER 1										€ PIER 2					€ N. ABUT. BRG.							
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23		24	25	26	27	28	29	30
ANTICIPATED DEFLECTION DUE TO SLAB AND SDL (mm)	A,K	0.000	-9.646	-17.023	-20.700	-20.191	-15.937	-9.325	-3.888	0.000	0.959	-0.471	-1.906	-2.302	-1.258	0.920	3.276	4.773	4.208	0.000	-8.409	-19.794	-32.535	-45.845	-56.844	-63.848	-65.777	-62.089	-52.822	-38.536	-20.374	0.000
	B,J	0.000	-10.694	-18.883	-22.985	-22.457	-17.774	-10.452	-4.395	0.000	1.262	-0.071	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---
	C THRU I	0.000	-11.741	-20.743	-25.270	-24.722	-19.610	-11.579	-4.901	0.000	1.565	0.329	-0.968	-1.103	0.342	2.926	5.522	6.946	5.742	0.000	-10.879	-25.401	-41.558	-58.374	-72.221	-80.593	-83.343	-78.604	-66.830	-48.733	-25.759	0.000
CROSS SLOPE ADJUSTMENTS (mm)	A,K,F	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	
	B	3.000	VARIES	VARIES	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---
	J	3.000	3.000	3.000	VARIES	VARIES	0.000	0.000	0.000	0.000	0.000	0.000	0.000	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---	---
	C	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	VARIES	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	D,E,G,H	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800	3.800
I	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000	3.000
ALLOWABLE FIELD HAUNCH (mm)	MAX.	50.0																														
	MIN.	0.0																														



NOTES:

- TO CALCULATE FIELD HAUNCH NEEDED AT EACH LOCATION, SURVEY THE BEAM TOPS CONSISTENT WITH THE SPACINGS SHOWN ON THE "TOP OF SLAB AND BEAM LINE HAUNCH ELEVATION LAYOUT" DIAGRAM ON DESIGN SHEET 35. SUBTRACT THE SURVEYED BEAM SHOT FROM THE "BEAM LINE HAUNCH ELEVATION". THIS VALUE WILL BE THE HAUNCH NEEDED (SEE "FIELD HAUNCH" IN HAUNCH DETAIL). THE "BEAM LINE HAUNCH ELEVATION" INCLUDES ADJUSTMENTS FOR SLAB THICKNESSES AND ANTICIPATED DEFLECTIONS. NO ADDITIONAL CALCULATIONS ARE REQUIRED. IF THE FIELD HAUNCH EXCEEDS THE MAXIMUMS AND MINIMUMS INDICATED IN THE MISCELLANEOUS DATA TABLE, ADJUSTMENTS TO THE GRADE OR ADDITIONAL HAUNCH REINFORCEMENT WILL BE REQUIRED.
- BRIDGE SEAT ELEVATIONS ARE SET BASED ON THEORETICAL CAMBER AND BEAM DEFLECTIONS. THESE BRIDGE SEATS WILL PROVIDE A THEORETICAL BEAM HAUNCH WITHIN DESIGN PARAMETERS. FIELD HAUNCHES ARE DETERMINED USING SURVEYED TOP OF BEAM ELEVATIONS AND "BEAM LINE HAUNCH ELEVATION" DATA. ALLOWABLE MAXIMUM AND MINIMUM "FIELD HAUNCH" VALUES ARE GIVEN IN THE "MISCELLANEOUS DATA" TABLE. "CROSS SLOPE ADJUSTMENT" VALUES WILL AID THE CONTRACTOR IN DETERMINING ACTUAL FORMED HAUNCH DIMENSIONS AT THE EDGES OF THE TOP FLANGE.

DESIGN FOR 6°35'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
SUPERSTRUCTURE DETAILS
 STATION : 20029+85.670 (€ 9th ST.) OCTOBER 2005
 STATION : 529+85.670 (€ 1-235)
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 30 OF 49 FILE NO. 28552 DESIGN NO. 2406

REINFORCING BAR LIST-ONE SUPER.&TWO ABUTS.

REINFORCING BAR LIST-ONE SUPER.&TWO ABUTS.

H. P. STRUCTURAL CONCRETE PLACEMENT QUANTITIES ONE SUPER.

MARK	SIZE	LOCATION	SHAPE	NO.	LENGTH	MASS
* a1	20	SLAB, TOP TRANSV.	—	193	VARIES	6 636
a2	20	SLAB, TOP TRANSV.	—	264	13 000	8 082
* a3	20	SLAB, TOP TRANSV.	—	192	VARIES	5 471
a4	20	SLAB, TOP TRANSV.	—	263	10 100	6 256
* a5	20	SLAB, BOTTOM TRANSV.	—	192	VARIES	6 669
a6	20	SLAB, BOTTOM TRANSV.	—	264	13 200	8 207
* a7	20	SLAB, BOTTOM TRANSV.	—	191	VARIES	5 533
a8	20	SLAB, BOTTOM TRANSV.	—	263	10 400	6 441
a9	15	SIDEWALK, TRANSVERSE	—	636	2 250	2 247
a10	15	SIDEWALK, TRANSVERSE	—	636	1 000	999
a11	15	SIDEWALK, TRANSVERSE	—	636	1 000	999
* a12	20	SLAB, TOP TRANSV.	—	42	VARIES	1 301
* a13	20	SLAB, TOP TRANSV.	—	44	VARIES	1 072
* a14	20	SLAB, BOTTOM TRANSV.	—	42	VARIES	1 320
* a15	20	SLAB, BOTTOM TRANSV.	—	44	VARIES	1 098
b1	20	SLAB LONGIT. TOP & BOTTOM	—	906	16 540	35 290
b2	15	SIDEWALK, LONGITUDINAL	—	96	16 740	2 478
d4	15	PAVING NOTCH LONGIT.	—	2	15 000	47
d6	15	ABUT. DIAPHRAGM LONGIT. B.F.	—	5	7 900	62
d7	15	ABUT. DIAPHRAGM LONGIT. B.F.	—	5	10 600	83
d9	15	PAVING NOTCH LONGIT.	—	2	11 800	37
d10	15	ABUT. DIAPH. LONGIT. B.F.	—	20	3 500	110
d11	15	ABUT. DIAPH. LONGIT. B.F.	—	5	13 900	109
d13	15	ABUT. DIAPH. LONGIT. B.F.	—	5	10 400	82
d15	15	ABUT. DIAPH. AT FORMED HOLE	—	2	2 430	31
f1	25	ABUT. FOOTING LONGIT. B.F.	—	8	12 300	386
f3	25	ABUT. FOOTING LONGIT. B.F.	—	8	15 400	484
f7	25	ABUT. FOOTING LONGIT.	—	4	16 100	253
f8	25	ABUT. FOOTING LONGIT.	—	4	10 470	164
g1	25	ABUT. VERT. B.F.	—	163	2 600	1 663
g3	25	ABUT. DIAPH. VERT. B.F.	—	148	5 100	2 863
h1	15	HEADER DIAPH.	—	16	900	23
h2	15	HEADER DIAPH.	—	4	1490	9
k1	15	PAVING NOTCH TRANSV.	—	139	1 500	327
k2	15	PAVING NOTCH TRANSV.	—	155	1 030	251
k3	15	PAVING NOTCH TRANSV. AT BAY E-F.	—	16	1 500	38
p1	15	ABUT. HOOPS	—	294	3 260	1 505
p3	20	ABUT. BOTT. AT PILES	—	42	2 030	201
t2	15	ABUT. DIAPH. LONGIT. F.F.	—	80	2 230	280
w1	15	ABUT. FOOTING TRANSV.	—	90	1 820	257
BARRIER RAIL - SEE DESIGN SHT. NO. 37						5 860
SEPARATION BARRIER RAIL - SEE DESIGN SHT. NO. 38						7 901
LIGHT POLE ANCHOR - SEE DESIGN SHT. NO. 40						82
REINFORCING STEEL EPOXY COATED - TOTAL (kg)						123 307

MARK	SIZE	LOCATION	SHAPE	NO.	LENGTH	MASS
d1	15	ABUT. DIAPHRAGM LONGIT. F.F.	—	20	3 380	106
d2	15	ABUT. DIAPHRAGM LONGIT. F.F.	—	5	10 350	81
d3	15	ABUT. DIAPHRAGM LONGIT. F.F.	—	5	7 500	59
d5	15	ABUT. DIAPHRAGM ENDS	—	20	1 450	46
d8	15	ABUT. DIAPH. EXTENSION LONGIT.	—	48	3 060	231
d12	15	ABUT. DIAPH. LONGIT. F.F.	—	5	13 630	107
d14	15	ABUT. DIAPH. LONGIT. F.F.	—	5	10 350	81
e1	15	SHAFT SPIRAL	—	25	77530	3043
L22x22x3.2 SPIRAL SPACER (1.0 Kg/m)						100 6 505 651
f2	25	ABUT. FOOTING LONGIT. F.F.	—	10	12 100	475
f4	25	ABUT. FOOTING LONGIT. F.F.	—	10	15 200	597
f5	25	ABUT. FOOTING EXTENSION LONGIT.	—	28	3 880	426
f6	25	ABUT. FOOTING EXTENSION LONGIT.	—	4	3 840	60
f9	30	SHAFT LONGITUDINAL	—	250	6 605	9 074
REINFORCING STEEL - TOTAL (kg)						17 339
g2	25	ABUT. VERT. F.F.	—	146	2 310	1 324
g4	20	ABUT. WING EXTENSION VERT.	—	88	2 830	586
p2	15	ABUT. FOOTING EXTENSION HOOPS	—	48	3 260	246
t1	15	UNDER GIRDERS AT ABUTMENT	—	20	1 300	41
PILE SPIRAL SIZE NO. W5 WIRE (3 kg/SPIRAL)						25 12 000 75
SPIRAL SP. L22 x 22 x 3.2 (0.6 kg/SPACER)						50 560 30
REINFORCING STEEL - TOTAL (kg)						17 339

SLAB SECTION	WEST	EAST	CLOSURE	QUANTITY
SLAB SECTION 1 & ABUT. DIAPH.	84.9	73.4	-	158.3
SLAB SECTION 2	31.4	24.5	-	55.9
SLAB SECTION 3 & ABUT. DIAPH.	92.2	73.2	-	165.4
SLAB SECTION 4	31.8	25.5	-	57.3
SLAB SECTION 5	52.4	40.3	-	92.7
SLAB SECTION 6	-	-	25.7	25.7
SIDEWALKS	-	-	-	75.3
TOTAL (m³)				630.6

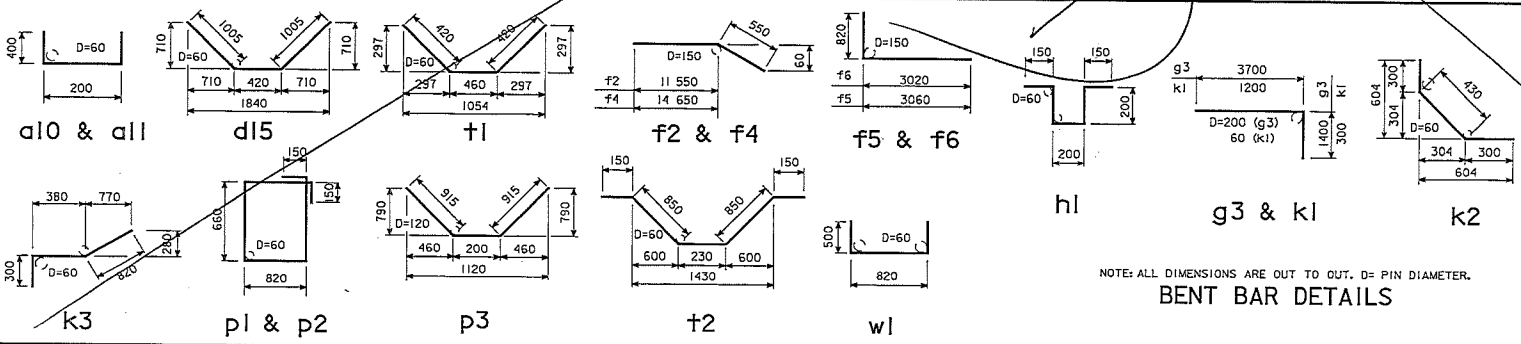
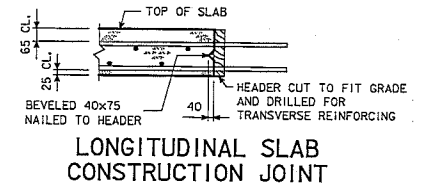
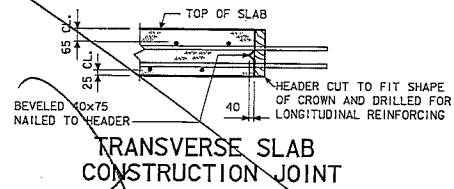
CONC. PLACEMENT QUANTITIES	TWO ABUTMENTS	
SOUTH ABUTMENT FOOTING	36.4	
NORTH ABUTMENT FOOTING	28.3	
TOTAL (m³)		64.7

ESTIMATED QUANTITIES	ONE SUPER. & TWO ABUTS.		
ITEM	UNIT	QUANTITY	
HIGH PERFORMANCE STRUCTURAL CONCRETE	m³	630.6	
STRUCTURAL CONCRETE (BRIDGE)	m³	64.7	
STRUCTURAL STEEL	Kg	297 876	
REINFORCING STEEL	Kg	17 339	
REINFORCING STEEL EPOXY COATED	Kg	123 307	
CLASS 20 EXCAVATION	m³	128	
HP310x125 STEEL BEARING PILES	FURNISH 12 @ 8.2 m N.A.; 13 @ 8 m S.A.	m	202.4
PILE CASING, CORRUGATED METAL PIPE, 600 mm		m	142.4
CONCRETE DRILLED SHAFT, 760 DIA.		m	168.3

EPOXY COATED REINFORCING

NON-COATED REINFORCING

- * a1 VARIES FROM 13 300 TO 15 900
- a3 VARIES FROM 10 600 TO 13 600
- a5 VARIES FROM 13 500 TO 16 000
- a7 VARIES FROM 10 800 TO 13 800
- a12 VARIES FROM 13 000 TO 13 300
- a13 VARIES FROM 10 100 TO 10 600
- a14 VARIES FROM 13 200 TO 13 500
- a15 VARIES FROM 10 400 TO 10 800

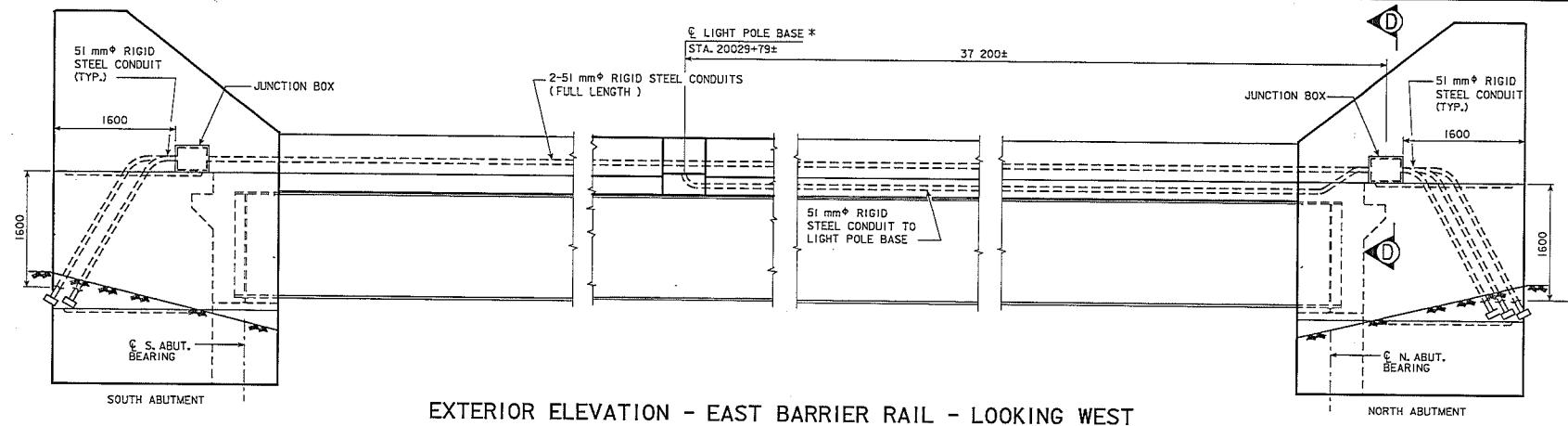


DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.380m, 31.600, 37.100 SPANS
SUPERSTRUCTURE DETAILS
 STATION : 20029+85.670 (E 34th ST.)
 STATION : 529+85.670 (E 1-285) OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 34 OF 49 FILE NO. 29552 DESIGN NO. 2406

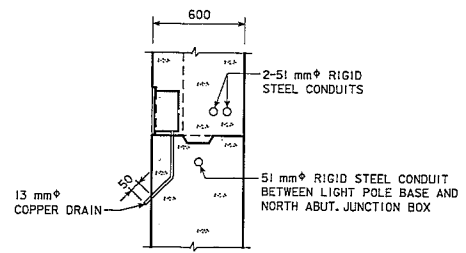
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CONDUIT PLACEMENT SUMMARY	
ITEM	AMOUNT
51 mm ϕ RIGID STEEL, WEST BARRIER	202 m
51 mm ϕ RIGID STEEL, EAST BARRIER	245 m
89 mm ϕ RIGID STEEL, PIER 2	6.4 m
38 mm ϕ RIGID STEEL, PIER 2	12 m

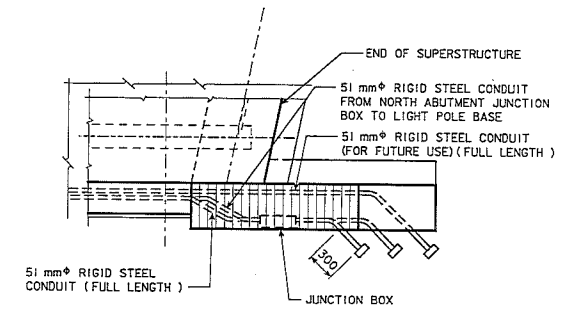
* NOTE:
ADJUST THE GIVEN STATION LOCATION AS NECESSARY
TO ALIGN THE LIGHT POLE BASE WITH THE CENTERLINE
OF THE PIER BELOW.



EXTERIOR ELEVATION - EAST BARRIER RAIL - LOOKING WEST



PART SECTION D-D



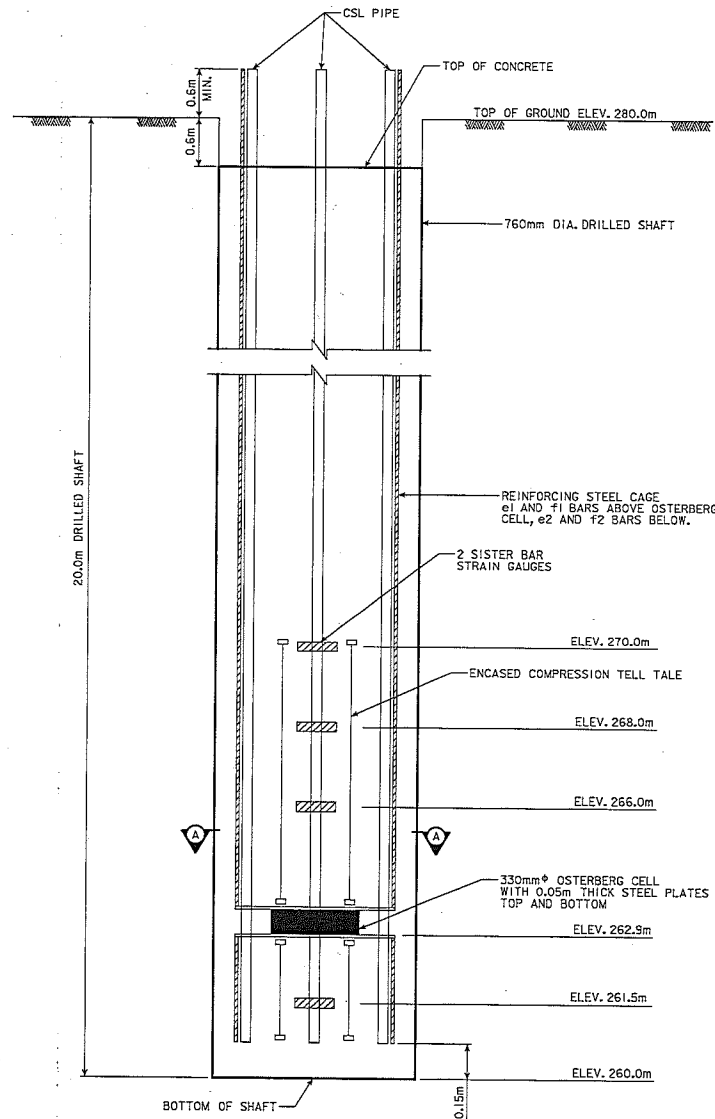
PART PLAN AT END POST

NOTES:
SEE DESIGN SHEET 36 FOR CONDUIT IN WEST BARRIER RAIL.
SEE DESIGN SHEET 40 FOR JUNCTION BOX DETAILS.

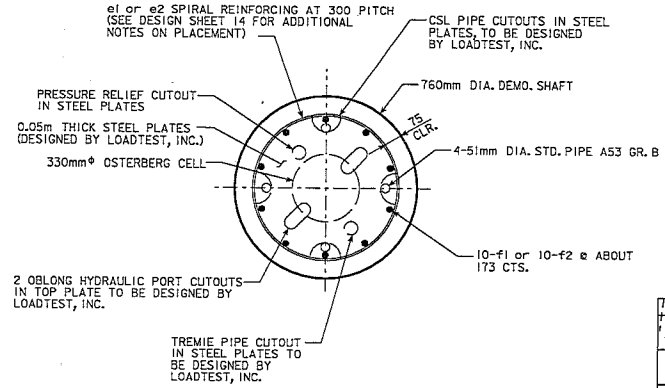
DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
LIGHTING DETAILS
 STATION : 20029+85.670 (C 9th ST.)
 STATION : 529+85.670 (C 1-235)
 OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 39 OF 49 FILE NO. 23552 DESIGN NO. 2406

8/15/2005 1:53:33 PM
 HM1030A.S01 : THIS SHEET ISSUED 9-1-95.
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DESIGNED BY MBO	CHECKED BY DL	LIGHTING	STANDARD SHEET M1030A (MODIFIED)	POLK COUNTY	PROJECT NUMBER	IM-235-2(313)8--13-77	SHEET NUMBER 40
DETAILED BY FTE	CADD FILE R772406.dwg						



DEMOSNTRATION SHAFT ELEVATION

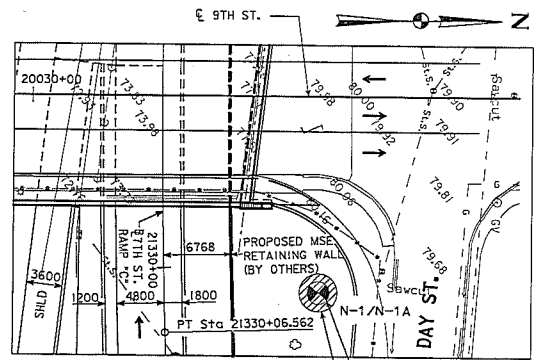


DEMOSNTRATION SHAFT PLAN SECTION A-A

REINFORCING BAR LIST - ONE DEMO. SHAFT							
MARK	SIZE	LOCATION	SHAPE	NO.	LENGTH	MASS	
f1	30	SHAFT LONGITUDINAL	—	10	17350	953	
e1	15	SHAFT SPIRAL	—	1	115285	181	
		L22x22x3.2 SPIRAL SPACER (1.0 kg/m)	—	4	17350	69	
f2	30	SHAFT LONGITUDINAL	—	10	2700	148	
e2	15	SHAFT SPIRAL	—	1	22684	36	
		L22x22x3.2 SPIRAL SPACER (1.0 kg/m)	—	4	2700	11	
						TOTAL NON-COATED (kg)	1398

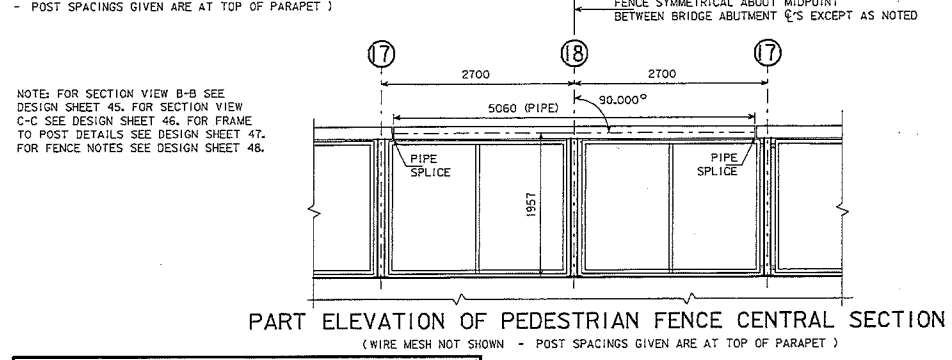
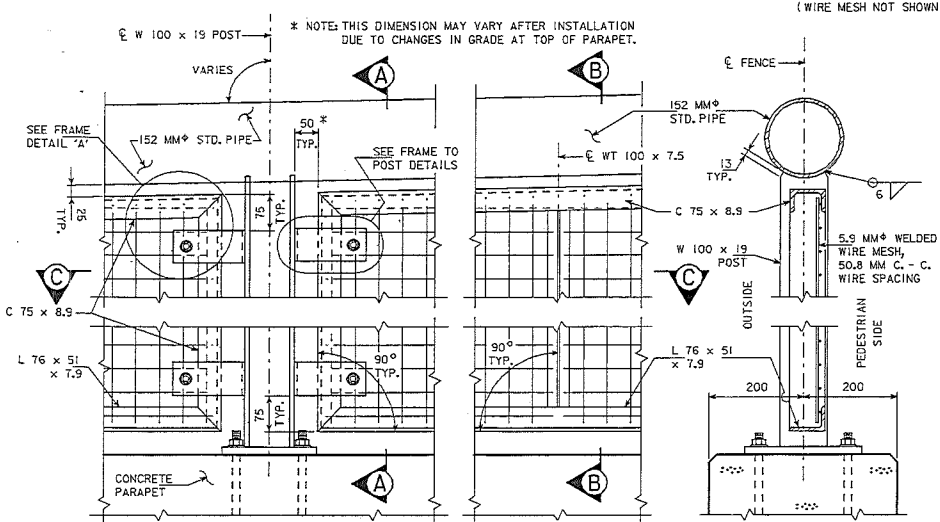
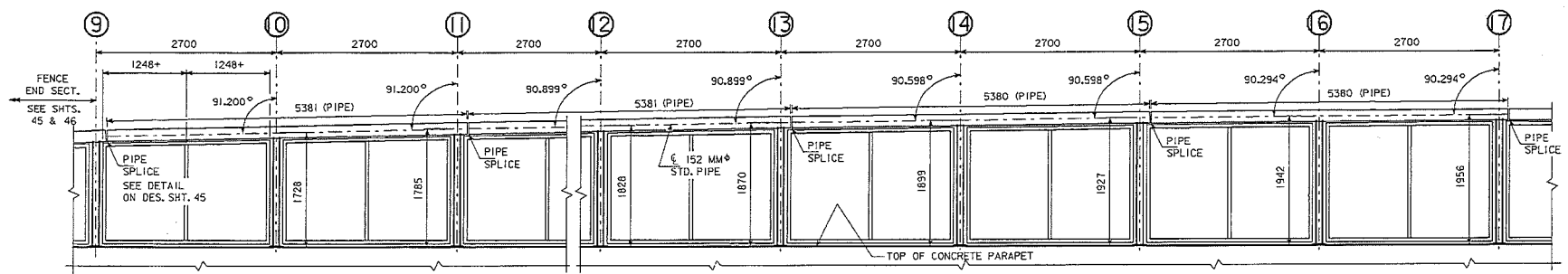
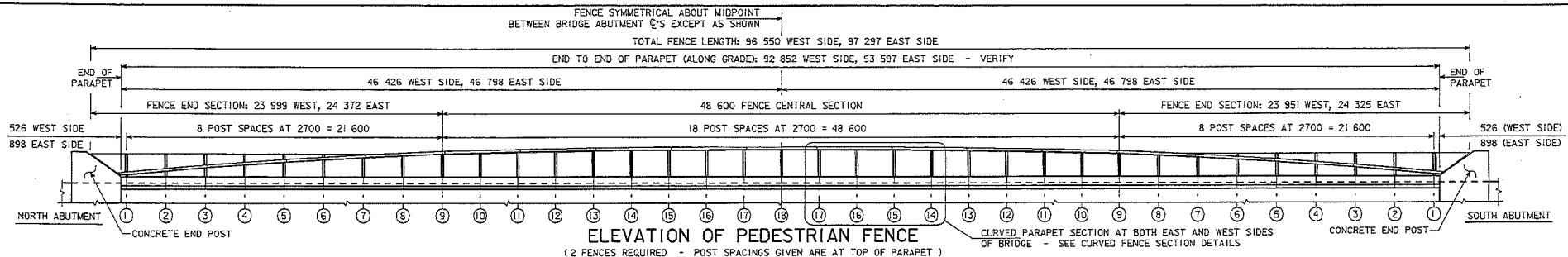
DEMOSNTRATION SHAFT NOTES:

- DRAWING IS NOT TO SCALE: FOLLOW DIMENSIONS.
- THE NOMINAL CAPACITY OF THE 330mm DIA. OSTERBERG CELL SHALL BE 4350 kN IN EACH DIRECTION.
- THE OSTERBERG CELL SHALL HAVE A MINIMUM STROKE OF 100mm.
- STRAIN GAUGES AND COMPRESSION TELL TALES ARE REQUIRED TO MONITOR THE PERFORMANCE OF THE LOAD TEST. STRAIN GAUGE AND ENCASED COMPRESSION TELL TALE LOCATIONS ARE PRELIMINARY AND ARE TO BE FINALIZED BY LOADTEST, INC. FINAL LOCATIONS ARE TO BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT.
- A COMPLETE COPY OF THE BORING LOG AND TEST RESULTS ARE AVAILABLE UPON WRITTEN REQUEST TO THE CONTRACTING AUTHORITY.
- THE ILLUSTRATED LOAD TEST DESIGN IS PRELIMINARY.
- FINAL DESIGN OF THE DEMOSNTRATION SHAFT SHALL BE SUBJECT TO CHANGE BY LOADTEST, INC. WITH THE APPROVAL OF THE ENGINEER.
- TREMIE PIPE SHALL BE LOWERED IN THE HOLE AT THE SAME TIME AS THE REINFORCING CAGE AND THE OSTERBERG LOAD CELL TO ENSURE THE TREMIE PIPE WILL EXTEND TO THE BOTTOM OF THE HOLE FOR CONCRETE PLACEMENT.



SITUATION PLAN

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
DEMOSNTRATION SHAFT
 STATION : 20029+85.670 (€ 9th ST.)
 STATION : 529+85.670 (€ I-235)
 OCTOBER 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 41 of 49 FILE NO. 29552 DESIGN NO. 2406



DESIGNED BY JRH CHECKED BY RJM
 DETAILED BY KMD CADD FILE H772406_S44

STRUCTURAL DESIGN

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

William D. Tucker 6-16-2005
 Signature Date
 Printed or Typed Name
 My license renewal date is December 31, 2005

Pages or sheets covered by this seal: SHEETS 44 THRU 49 OF 49

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
FENCE DETAILS
 STATION : 20029+85.670 (E 9th ST.)
 STATION : 529+85.570 (E 1-235)
 OCTOBER, 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 44 OF 49 FILE NO. 29552 DESIGN NO. 2406

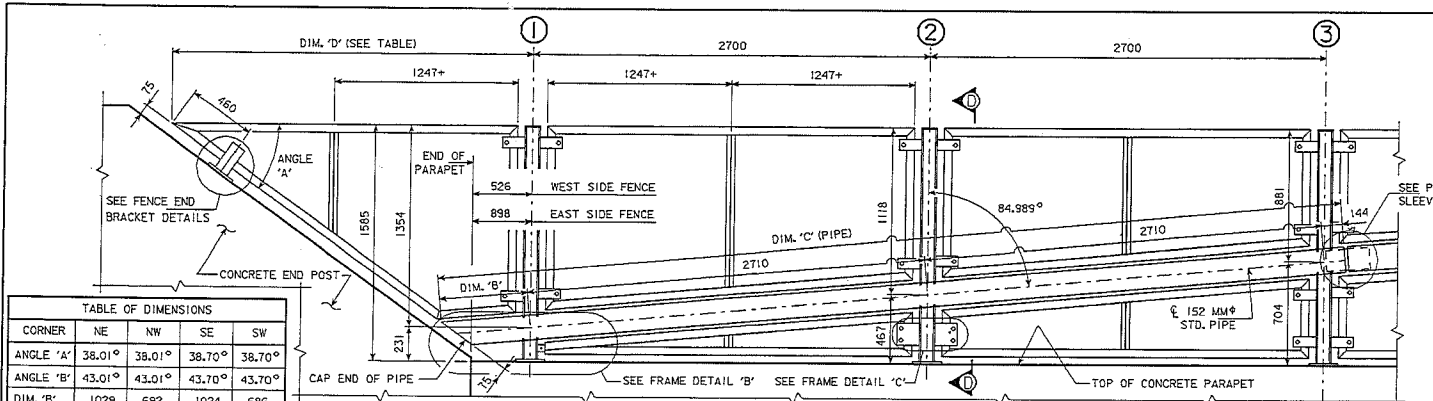
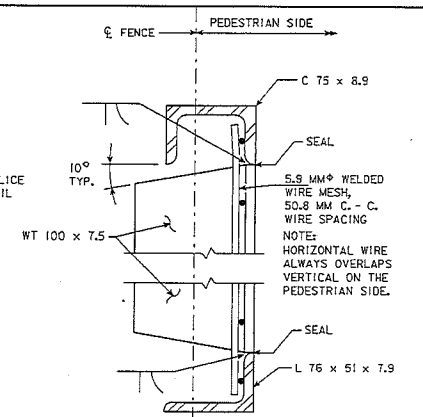
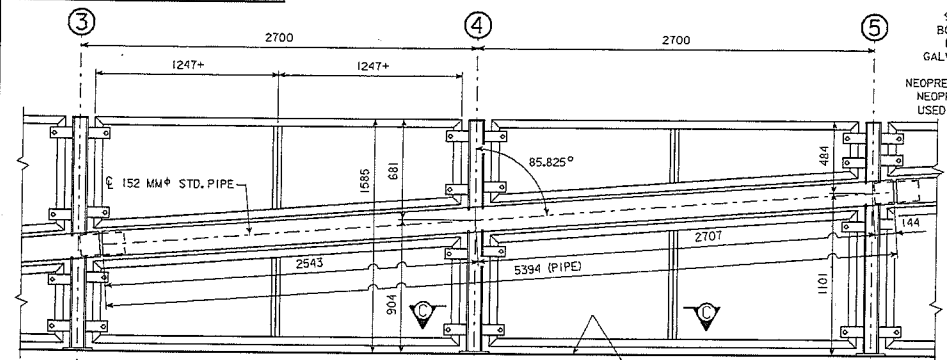


TABLE OF DIMENSIONS				
CORNER	NE	NW	SE	SW
ANGLE 'A'	38.01°	38.01°	38.70°	38.70°
ANGLE 'B'	43.01°	43.01°	43.70°	43.70°
DIM. 'B'	1029	692	1024	686
DIM. 'C'	6593	6256	6588	6250
DIM. 'D'	2772	2399	2725	2351

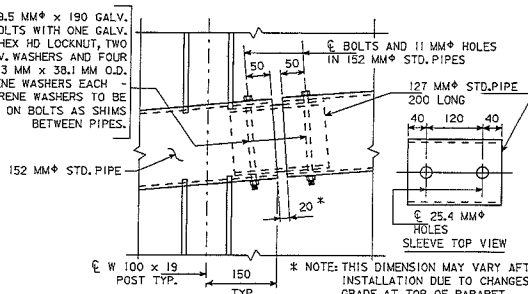
PART ELEVATION - PEDESTRIAN FENCE END SECTION
(OUTSIDE OF FENCE SHOWN - WIRE MESH NOT SHOWN - POST SPACINGS GIVEN ARE AT TOP OF PARAPET)



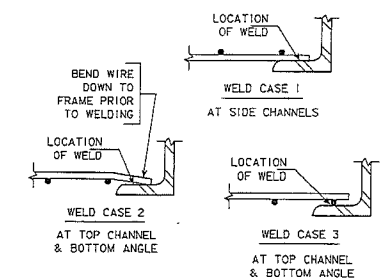
PART SECTION B - B
NOTE: FOR LOCATION OF VIEW SEE SHEETS 1 & 3.



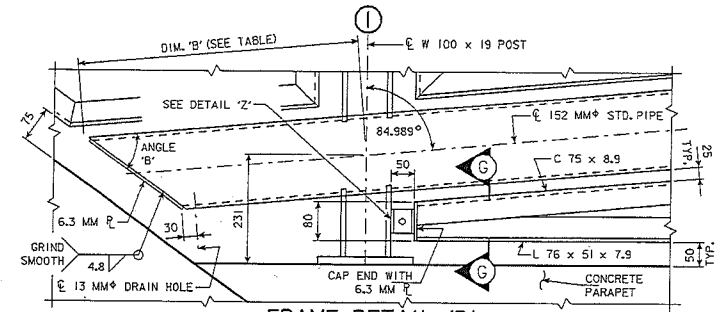
PART ELEVATION - PEDESTRIAN FENCE END SECTION
(OUTSIDE OF FENCE SHOWN - WIRE MESH NOT SHOWN - POST SPACINGS GIVEN ARE AT TOP OF PARAPET)



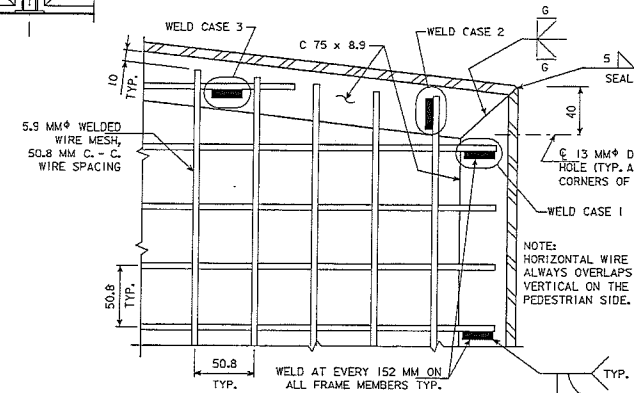
PIPE SPLICE SLEEVE DETAIL
(MESH PANEL FRAMES NOT SHOWN)



WIRE MESH WELD CASE DETAILS
SEE FRAME DETAIL 'A' FOR LOCATIONS
NOTE: DO NOT WELD BOTH ENDS OF THE SAME WIRE TO THE FRAME.



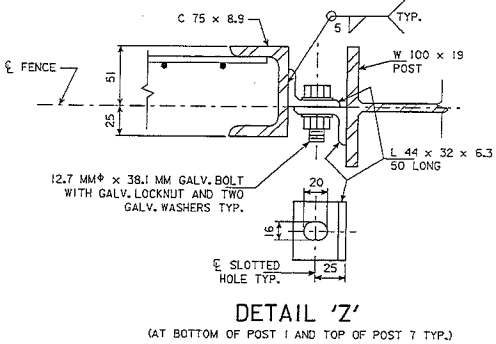
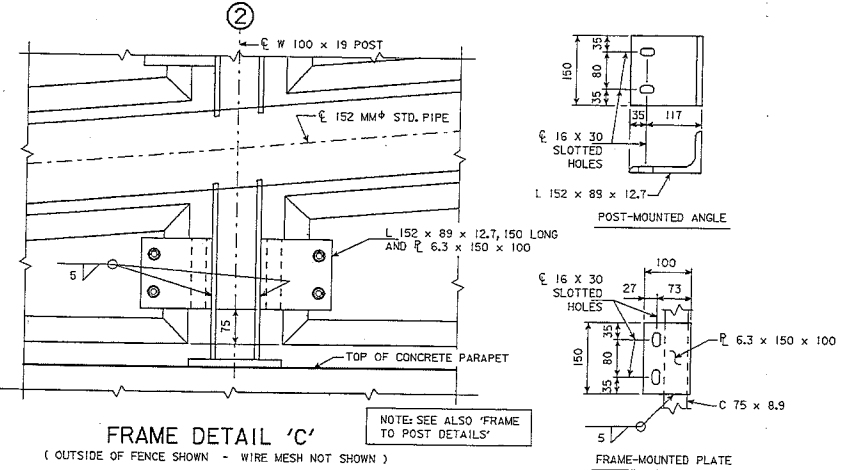
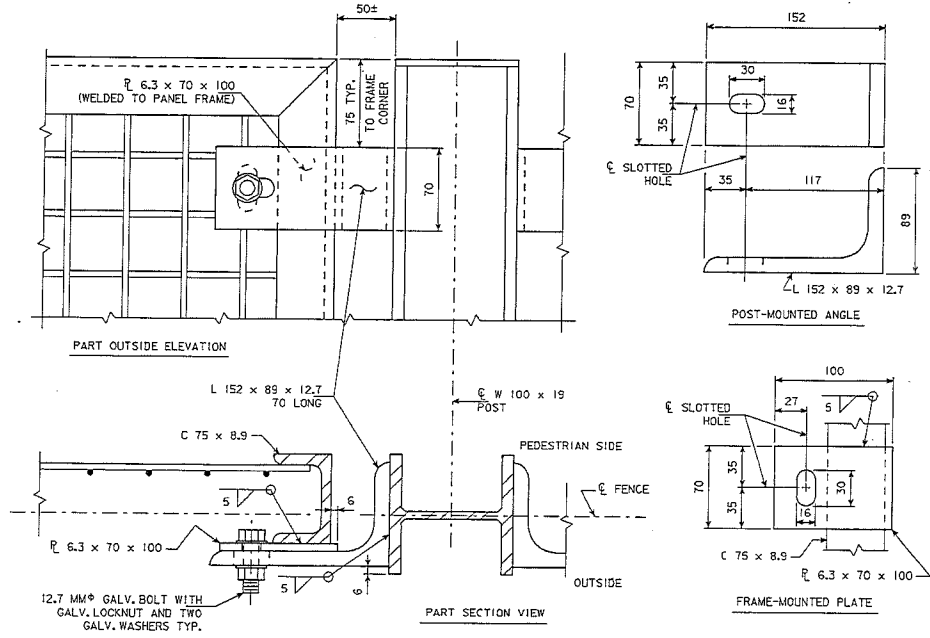
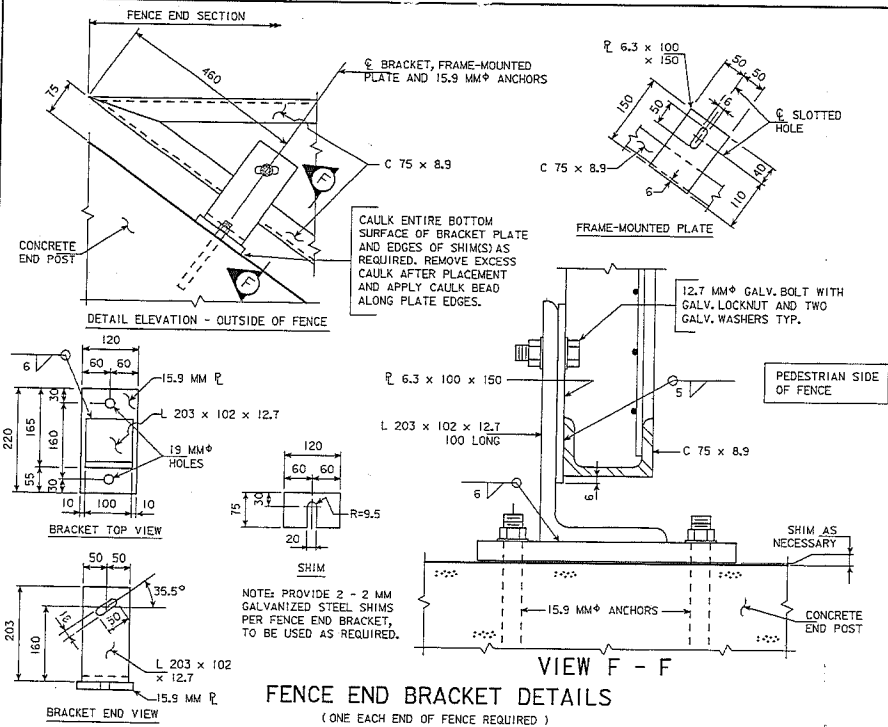
FRAME DETAIL 'B'
(OUTSIDE OF FENCE SHOWN - WIRE MESH NOT SHOWN)



FRAME DETAIL 'A'
(BACK SIDE SHOWN - BACK FLANGE OF CHANNELS NOT SHOWN)

NOTE: FOR SECTION VIEWS C-C AND D-D SEE DESIGN SHEET 46.
FOR LOCATION OF SECTION VIEW B-B SEE DESIGN SHEETS 44 AND 46.
FOR VIEW G-G AND FENCE NOTES SEE DESIGN SHEET 48.
FOR FRAME DETAIL 'C' AND DETAIL 'Z' SEE DESIGN SHEET 47.

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
25.300m, 31.600, 37.100 SPANS
FENCE DETAILS
STATION : 20029+85.670 (E 3rd ST.)
STATION : 529+85.570 (E 1-235)
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET No. 45 OF 49 FILE No. 29552 DESIGN No. 2406



DESIGN FOR 6°36'00" SKEW (L.A.)

94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK

25.300m, 31.600, 37.100 SPANS

FENCE DETAILS

STATION : 20029+85.670 (E 9th ST.)

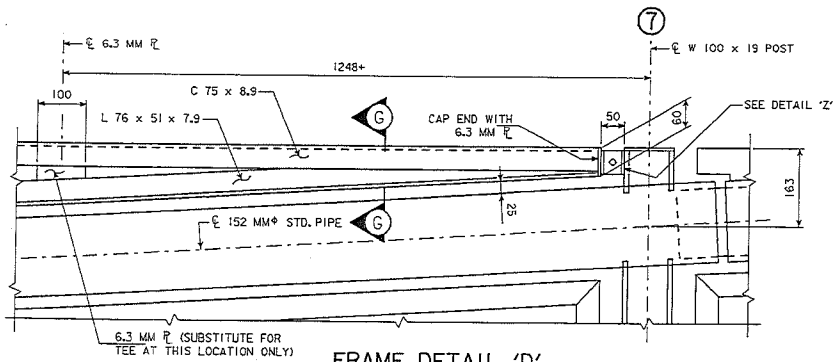
STATION : 529+85.570 (E 1-235)

OCTOBER, 2005

POLK COUNTY

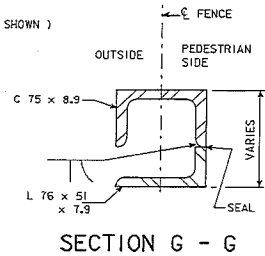
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

DESIGN SHEET NO. 47 of 49 FILE NO. 29562 DESIGN NO. 2406

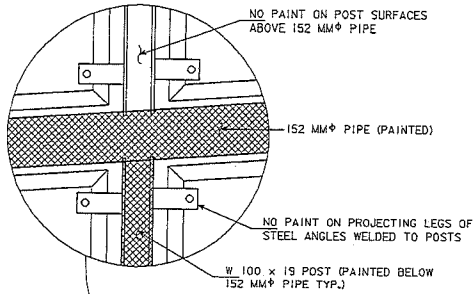


FRAME DETAIL 'D'

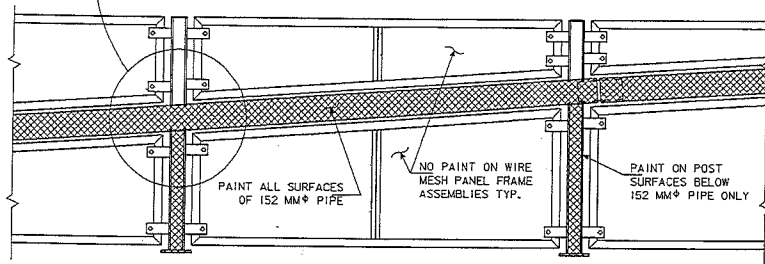
(OUTSIDE OF FENCE SHOWN - WIRE MESH NOT SHOWN)



SECTION G - G



PAINTING DETAILS



FENCE NOTES:

ALL FRAME ASSEMBLIES ARE TO BE SET NORMAL TO GRADE.

CONTRACTOR SHALL VERIFY DIMENSIONS OF CONCRETE PARAPET AND END POSTS ON BRIDGE PRIOR TO COMMENCING FINAL LAYOUT AND INSTALLATION OF FENCE. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IN CONCRETE DIMENSIONS PRIOR TO FENCE INSTALLATION.

ANCHORAGES SHALL BE ACCURATELY PLACED TO PROVIDE CORRECT ALIGNMENT OF FENCE. ANCHORS ARE TO BE 15.9 MM DIAMETER THREADED RODS CONFORMING TO ASTM A 709M, GRADE 250. ANCHORS SHALL USE ONE OF THE FOLLOWING ANCHORING SYSTEMS:

1. HILTI HIT HY150/HIT-ICE WITH 190 MM MINIMUM EMBEDMENT DEPTH
2. SIMPSON STRONG-TIE ACRYLIC-TIE WITH 190 MM MINIMUM EMBEDMENT DEPTH
3. WEJ-IT INJECT-TITE WITH 240 MM MINIMUM EMBEDMENT DEPTH
4. APPROVED EQUAL

ALL ANCHORING HARDWARE IS TO BE GALVANIZED PER THE STANDARD SPECIFICATIONS.

STRUCTURAL STEEL POSTS SHALL COMPLY WITH ASTM A 709M, GRADE 345. STANDARD PIPE SHALL COMPLY WITH ASTM A 53M GRADE B TYPE E. ALL OTHER STRUCTURAL STEEL MATERIALS SHALL COMPLY WITH ASTM A 709M, GRADE 250 MINIMUM.

NO SINGLE WIRE IN THE WELDED WIRE MESH SHALL BE WELDED TO THE FRAME AT BOTH ENDS OF THE WIRE.

ALL BURRS AND SHARP CORNERS OF STEEL FENCE COMPONENTS SHALL BE GROUND SMOOTH PRIOR TO GALVANIZING AND PAINTING.

ALL STRUCTURAL STEEL TUBE, "W" POST AND BASE PLATE ASSEMBLIES AS SHOWN IN THE PLANS ARE TO BE PAINTED AFTER GALVANIZING IN ACCORDANCE WITH THE SUPPLEMENTAL SPECIFICATION "CLEANING, SURFACE PREPARATION AND PAINTING OF GALVANIZED SURFACES". PAINT COLOR IS TO MATCH FEDERAL STANDARD COLOR NO. 25052 (BLUE). PAINT SHALL BE EXCLUDED FROM SURFACES OF STEEL ANGLE PANEL MOUNTING TABS AND PORTIONS OF POSTS ABOVE THE 152 MM DIA. PIPE AS SHOWN IN THE PLANS BY MEANS OF MASKING. PAINT EDGE SHALL BE ALONG CLEAN, STRAIGHT LINES AT MASKED SURFACES.

WIRE MESH PANELS AND ASSOCIATED CHANNEL AND ANGLE FRAME ASSEMBLIES SHALL NOT BE PAINTED, BUT SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. ALL FENCE HARDWARE SHALL BE GALVANIZED PER THE STANDARD SPECIFICATIONS.

THE WELDED WIRE MESH PANELS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A 123M. GOOD STANDARD PRACTICES SHALL BE FOLLOWED IN ACCORDANCE WITH ASTM A 143 AND ASTM A 384. PREPARE THE FABRICATED PANEL SURFACES BY ABRASIVE BLAST CLEANING TO A MINIMUM OF SSPC-SP 6 "COMMERCIAL BLAST CLEANING" PRIOR TO HOT-DIP GALVANIZING. THE HOT-DIP GALVANIZED PANELS WILL BE PROCESSED UTILIZING A "DRY" KETTLE.

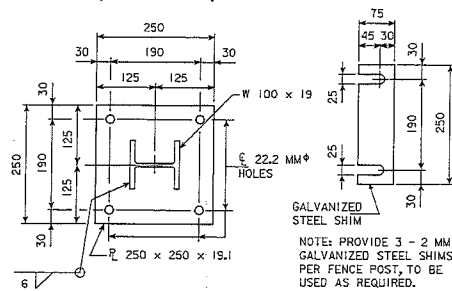
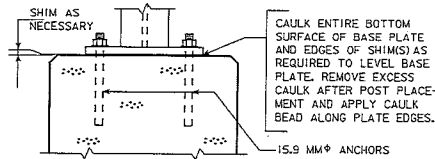
PANELS WILL BE PREFLUXED PRIOR TO THE GALVANIZING BATH USING AN AQUEOUS TANK OF ZINC CHLORIDE/AMMONIUM CHLORIDE. THE USE OF A "TOP FLUX" BLANKET ON THE MOLTEN ZINC BATH WILL NOT BE PERMITTED. WHEN IMMERSING PANELS IN THE GALVANIZING BATH, ONLY A FULL SINGLE DIP IS ALLOWED, WITH PANELS ORIENTED SUCH THAT ALL SURFACES OF THE ASSEMBLY RECEIVE THE ZINC COATING, WITH NO AIR BUBBLES OR VOIDS AT ANY SURFACE OF THE ASSEMBLY. NO MULTIPLE DIPPING OF PANELS WILL BE ALLOWED. UPON REMOVAL FROM THE GALVANIZING BATH, PANELS SHALL BE MANIPULATED IN ORDER TO PROPERLY DRAIN AWAY ALL EXCESS GALVANIZING SOLUTION. AIR COOL PANELS TO AMBIENT TEMPERATURE BEFORE HANDLING FOR SHIPMENT AND/OR STORAGE. DO NOT QUENCH PANELS OR APPLY ANY POST-GALVANIZING TREATMENTS. CALIBRATE DRY FILM THICKNESS GAGES IN ACCORDANCE WITH SSPC-PA 2 (CURRENT EDITION). WHEN GALVANIZED PANELS ARE TO BE STORED AND OUTDOOR STACKING IS UNAVOIDABLE, THE PANELS SHALL BE RAISED FROM THE GROUND AND PROPERLY SEPARATED TO PROVIDE FREE AIR ACCESS TO ALL PARTS OF THE SURFACE. THEY SHOULD ALSO BE INCLINED IN A MANNER WHICH WILL PROVIDE MAXIMUM DRAINAGE TO PREVENT THE FORMATION OF "WHITE-RUST" OR WET STORAGE STAINING. THE GALVANIZER SHALL PROVIDE TO THE ENGINEER ALL GALVANIZING PROCESS-RELATED QUALITY CONTROL DOCUMENTS INCLUDING, BUT NOT LIMITED TO, COATING MATERIAL CERTIFICATIONS, VISUAL EXAMINATIONS AND COATING THICKNESS EXAMINATIONS PRIOR TO FINAL ACCEPTANCE.

ALL PAINTED SURFACES SHALL BE PROTECTED IMMEDIATELY AFTER PAINT HAS CURED. PROTECTION METHOD SHALL BE ADEQUATE TO PREVENT DAMAGE TO THE PAINT DURING STORAGE, HANDLING, SHIPPING TO THE INSTALLATION SITE AND DURING THE INSTALLATION OF THE FENCE. PROTECTION SHALL NOT BE REMOVED UNTIL POTENTIAL DAMAGE TO THE PAINT IS LIMITED TO ASSEMBLY SURFACES ONLY. TOUCH-UP REPAIR OF DAMAGED PAINT IS TO BE IN ACCORDANCE WITH THE SUPPLEMENTAL SPECIFICATION "CLEANING, SURFACE PREPARATION AND PAINTING OF GALVANIZED SURFACES".

ALL FENCE MEMBERS SHALL BE FLAT AND STRAIGHT AFTER FABRICATION AND GALVANIZING TO WITHIN 3 MM IN 3 M BY MECHANICAL MEANS WITHOUT DAMAGE TO THE ZINC COATING.

CAULK FOR BASE PLATES SHALL BE WHITE NONSAG LATEX CAULK MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED. EXCESS CAULK SHALL BE COMPLETELY REMOVED FROM SURROUNDING CONCRETE SURFACES.

ALL COSTS ASSOCIATED WITH THE FENCE INCLUDING THE ANCHORAGES AND PAINTING SHALL BE INCLUDED IN THE PRICE BID FOR "STEEL FENCE, WELDED WIRE MESH".



POST BASE PLATE DETAILS

NOTE: FOR LOCATION OF FRAME DETAIL 'D' SEE DESIGN SHEET 46.

QUANTITIES

ITEM	UNITS	AMOUNT
STEEL FENCE, WELDED WIRE MESH	M	193.8

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25,300m, 31,600, 37,100 SPANS
FENCE DETAILS
 STATION : 20029+85.670 (E 9th ST.)
 STATION : 529+85.570 (E 1-235) OCTOBER, 2005
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 48 OF 49 FILE NO. 29552 DESIGN NO. 2406

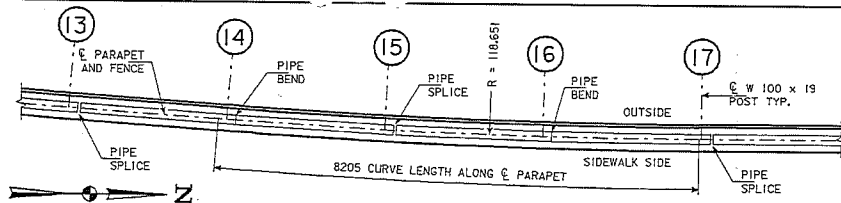
DESIGNED BY JHH CHECKED BY RUM
 DETAILED BY KMO CADD FILE H772406.348

POLK COUNTY

PROJECT NUMBER

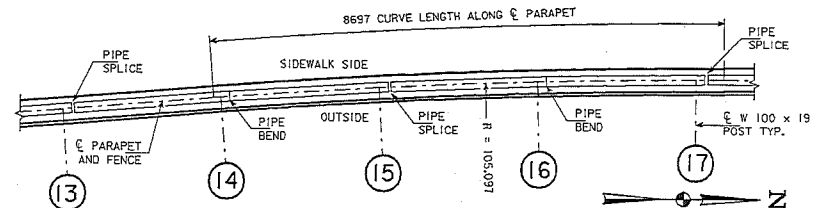
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SHEET NUMBER 49



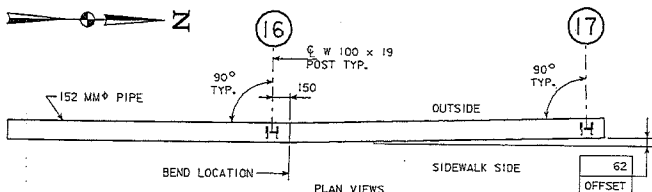
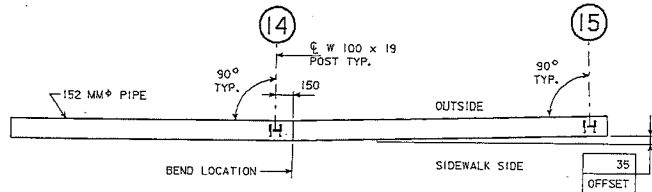
PART PLAN OF WEST FENCE NEAR BRIDGE CENTER

(MESH PANEL FRAMES NOT SHOWN)



PART PLAN OF EAST FENCE NEAR BRIDGE CENTER

(MESH PANEL FRAMES NOT SHOWN)



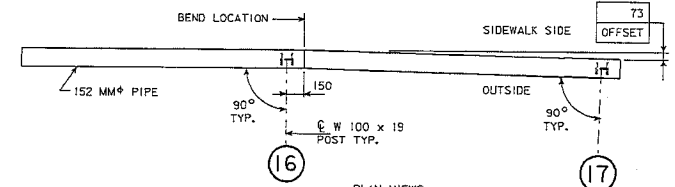
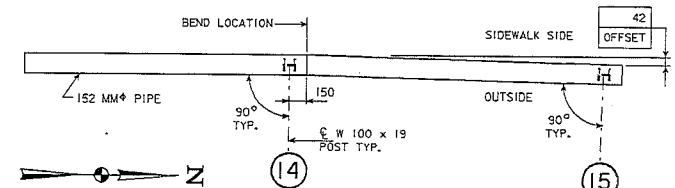
PLAN VIEWS

WEST FENCE PIPE BEND DETAILS

CURVED FENCE SECTION NOTES:

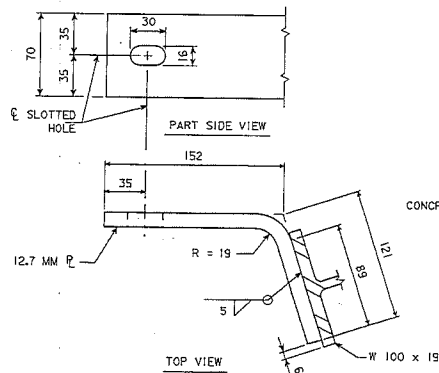
CURVED PARAPET OCCURS JUST SOUTH OF THE BRIDGE CENTER ON BOTH EAST AND WEST SIDES. SEE SITUATION PLAN AND OTHER DETAILS IN THESE PLANS FOR MORE INFORMATION.

ALL FENCE POSTS SHALL BE MOUNTED ON THE CENTERLINE OF CONCRETE PARAPET. FENCE SHALL HAVE STRAIGHT CHORDS AS SHOWN IN THE DETAILS, WITH CHANGES IN ALIGNMENT OF FENCE CENTERLINE TO OCCUR ONLY AT THE BENDS IN THE 152 MM PIPE, AT PIPE SPLICE LOCATIONS AND AT MESH FRAME MOUNTING ANGLES AT POSTS.

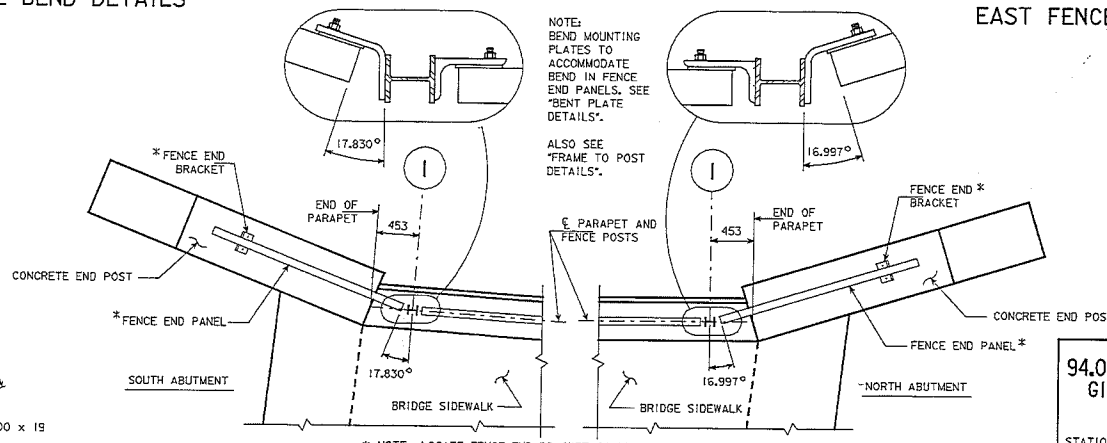


PLAN VIEWS

EAST FENCE PIPE BEND DETAILS



BENT PLATE DETAILS



PART PLAN AT WEST FENCE ENDS
(152 MM PIPE NOT SHOWN)

DESIGN FOR 6°36'00" SKEW (L.A.)
94.0 m x VARIABLE CONTINUOUS WELDED GIRDER BRIDGE W/ 2-2.4m SIDEWALK
 25.300m, 31.600, 37.100 SPANS
FENCE DETAILS
 STATION : 20029+85.670 (E 9th ST.)
 STATION : 529+85.570 (E I-235)
POLK COUNTY
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
 DESIGN SHEET NO. 49 OF 49 FILE NO. 29552 DESIGN NO. 2406

DESIGNED BY JRH CHECKED BY KMD
 DETAILED BY KMD CADD FILE H772406.S49

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POLK COUNTY

PROJECT NUMBER

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SHEET NUMBER

50